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CIVIL ENGINEERING LABORATORY Naval Construction Battalion Center Port Hueneme, California

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ENGINEERING REPORT ON WAVE TANK TESTS ON SPLIT PIPE

December 1977

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UNCLASSIFIED FICATION OF THIS PAGE (When Date Entered) READ INSTRUCTIONS BEFORE COMPLETING FOR REPORT DOCUMENTATION PAGE CR-78.007 ENGINEERING REPORT ON WAVE TANK TESTS December 1977 ON SPLIT PIPE. PERFORMING ORG. REPORT NUMBER CONTRACT OR GRAND . AUTHORIE Tokuo/Yamamoto N68305-77-C-0041 Ocean Engineering, Wave Research Facil-ity T.R.3, School of Engineering Oregon State University, Corvallis, OR PROGRAM ELEMENT, PROJECT, TASK YF52.556.091.01.316 Naval Facilities Engineering Command ¥ 3077 Dece 200 Stovall Street Alexandria, VA 22332 A MONITORING AGENCY NAME & ADDRESSE!! dillerent ! Civil Engineering Laboratory Unclassified Naval Construction Battalion Center SCHEDULE Port Hueneme, CA 93043 6. DISTRIBUTION STATEMENT (of this Report) Approved for public release; distribution unlimited 17. DISTRIBUTION STATEMENT (of the obstract entered in Block 20, if different to YF52556091 IR. SUPPLEMENTARY NOTES 19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Wave forces, split pipe, lift, drag, inertia 20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Laboratory wave tank tests were conducted to measure and record the horizontal and vertical wave forces on a prototype split pipe with nearly full scale design wave conditions. The ranges of the Reynolds number and the Keulegan-Carpenter number covered are  $10^4$  to  $2 \times 10^5$  and 0 to 40, respectively. The tests are done for three water depths, (4 feet, 6 feet and 8 feet), DD I JAN 73 1473 A EDITION OF I NOV 65 IS OBSOLETE UNCLASSIFIED SECURITY CLASSIFICATION OF THIS PAGE (When Date Entered) 88 06 08 012 10,000 +0 200,000

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three wave periods (2 sec. 4 sec and 6 sec), four wave heights and four orientations (0°, 45°, 90° and -45°), of the bolting flanges of the split pipe. The lift, drag, inertia and maximum horizontal force coefficients were evaluated based on the Airy wave theory and the Morrison equations and other wave force equations. The wave force coefficients are dependent on the Reynolds number, Keulegan-Carpenter number and the flange angle.

The single most important design parameter is determined to be the flange angle. When the flanges are parallel to the bottom, both horizontal and vertical forces are minimum, but the forces are increased by up to seven times when the flanges have large angles to the flow direction. Thus, the disorientation of the flanges by the waves may be a major contributor to split pipe failures.

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#### **ABSTRACT**

Laboratory wave tank tests were conducted to measure and record the horizontal and vertical wave forces on a prototype split pipe with nearly full scale design wave conditions. The ranges of the Reynolds number and the Keulegan-Carpenter number covered are  $10^4$  to  $2 \times 10^5$  and 0 to 40, respectively. The tests are done for three water depths, (4 feet, 6 feet and 8 feet), three wave periods (2 sec, 4 sec and 6 sec), four wave heights and four orientations (0°, 45°, 90° and -45°), of the bolting flanges of the split pipe. The lift, drag, inertia and maximum horizontal force coefficients were evaluated based on the Airy wave theory and the Morrison equations and other wave force equations. The wave force coefficients are dependent on the Reynolds number, Keulegan-Carpenter number and the flange angle.

The single most important design parameter is determined to be the flange angle. When the flanges are parallel to the bottom, both horizontal and vertical forces are minimum, but the forces are increased by up to seven times when the flanges have large angles to the flow direction. Thus, the disorientation of the flanges by the waves may be a major contributor to split pipe failures.

#### **ACKNOWLEDGEMENTS**

This work was done in accordance with the contract N68305-77-C-0041 between the Civil Engineering Laboratory of the Navy Department and Oregon State University. Mr. John Ciani of CEL provided valuable suggestions during the course of the project. Dr. John H. Nath of OSU joined with the author in developing the test program, suggested and designed the force dynamometers, and reviewed this report for submittal, with particular input to Section 10.0. Lt. Tim Brandenburg of the Navy Engineering Corps, as a Graduate Student, Mr. Larry Crawford and Mr. Terry Dibble, laboratory engineers, have assisted in the wave tank testing. Mr. Koji Kobune and Mr. M. C. Chen assisted in the data reduction.

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### 1.0 INTRODUCTION

### 1.1 Background Information and Problem Statement

The primary means used by the Navy for protecting and immobilizing submarine cables is split pipe. This pipe is shown in Figure 1 which illustrates the bolting together of the upper and lower half sections and the mating of the assembled full pipe sections. The approximate inside and ouside diameters of the main portion of the pipe are 3.5 in. (ID) and 5.0 in. (OD), which does not include the bell ends or bolting flanges.

Plain pipelines, which do not have the bell ends and flanged sides that characterize split pipe, are used extensively in engineered construction in the ocean by civilian and military organizations for oil and gas transport, sewage disposal and other common applications. Submarine cables for power and signal transmission are also widely used in industry and the military, but these are often protected by burial rather than split pipe.

Submarine pipelines must be designed to resist the hydrodynamic forces caused by waves which depend on the water particle accelerations and velocities. The hydrodynamic forces on pipelines are usually estimated by the Morrison equations or similar empirical equations with empirical coefficients of inertia, drag and lift. The force coefficients must be derived from laboratory or field experiments on pipe sections under the influence of waves or similar flow conditions. Virtually all of the past experiments (Ref. 1, 2, 4, 5, 6, 7, 8) of this type have involved plain circular cylinder shaped pipe but never split pipe. As a consequence, the coefficients derived from these tests are applicable to plain pipelines but not to split pipe because the flow around shapes other than plain cylinders is significantly different from that around plain pipe and therefore the forces are different. Thus, the force coefficients that are presently available for the design of "pipelines" cannot be applied to the design of lines of split pipe.

Furthermore, the bolting flanges of the split pipe will act like airfoils. The lift and drag forces on an airfoil are dramatically changed by the angle of attack. Although the flanges of the split pipe are usually laid horizontally at the installation, the flange angles may change due to the wave action. The increased wave forces, due to the re-orientation of the flange angle, may cause the failure of split pipelines. The quantitative information on the effect of the flange angle on the hydrodynamic forces on the split pipe has not been available.

### 1.2 Objectives

The purpose of this work is to measure and record horizontal and vertical wave forces on split pipe in a wave tank and reduce these data to give force coefficients of inertia, drag and lift for split pipe. Special attention is given to the effect of the flange orientation on the force coefficients.

### 1.3 Scope

At the Wave Research Facility at Oregon State University, wave tank tests are performed on the test section consisting of a three-section length of split pipe under various conditions of water depth, wave height and period, and inclination of the plane of the pipe flanges relative to the bottom of the tank (flange angle). The horizontal and vertical forces imposed by the waves on the test pipe section are measured and recorded along with the characteristics of the waves. Force coefficients of drag, inertia and lift are derived and reported. This work includes the design, fabrication and installation of the test equipment; the performance of the tests; the reduction of the data; and a separate report which includes the raw data.

### 2.0 TEST SCHEDULE AND MODIFICATIONS

The schedule of wave tank tests and the actual work done during the three weeks period of July 18 to August 5, 1977 are summarized in Table 1.

On July 21, 1977, during the installation of the test pipe force dynamometer unit at the wave tank, one of the two force dynamometers was accidentally bent slightly. This was immediately reported to CEL. Calibration of the two force dynamometers was made to see their response characteristics. It was found that both dynamometers gave excellent linear response in both horizontal and vertical directions. Only the bent force dynamometer showed a slight response to torque. (For a detailed discussion, read the calibration results in Section 6.3.) Since no critical damage to the force dynamometer was found, the experiments proceeded as scheduled.

The nine test combinations of the three water depths, h = 4', 6' and 8', and the three flange angles  $\phi$  = 0°, 45°, 90°, were originally scheduled. For each combination, twelve waves with the three wave periods, T = 2 sec. 4 sec, 6 sec, and the four wave heights were scheduled. For each combination of water depth and wave period, the clean maximum wave height H<sub>max</sub> was determined by trial and error. The values of H<sub>max</sub> are given in Table 3. Then the four wave heights were selected by taking 100, 75, 50 and 25% of H<sub>max</sub>. All of the scheduled 11 test combinations (h = 4', 6', 8' and  $\phi$  = 0°, 45°, 90° and two duplications) were completed ahead of schedule. Since some wave tank time was left, three more test combinations ( $\phi$  = -45° for h = 4', 6', 8') were made.  $\phi$  = -45° means that flange at the wave board side is down at an angle of 45°. Thus, a total of 14 test combinations, totaling 168 runs, were conducted.

Lt. Robert Steimer from CEL inspected the wave tank tests on August 2 and 3. He observed test series No. 12, 13,14 (Run No. 133 to 168) and the calibration of the force dynamometers after the tests.

#### 3.0 DESCRIPTION OF TESTING APPARATUS

## 3.1 OSU Wave Research Facility

The OSU Wave Research Facility is on an open site which is convenient to many types of research and is shown in Figure 2. The major unit is a wave and towing basin which is 104.27 m long (342'), 3.66 m wide (12') and generally 4.57 m deep (15'). Usually a 1 m (3.3') freeboard exists so that the water is 3.66 m (11.7') deep. The wave board is a flap-type board which is hinged at the bottom in a section which has a total depth of 5.49 m (18'). The board is activated by a 150 hp pump with a hydraulic servo mechanism which was designed and installed by MTS Systems Corporation of Minneapolis. The facility is the first to be built in the United States to have water on one side only of the wave board, which reduces the required power to activate it.

The facility has the capability of producing solitary waves, periodic waves and random waves which will model the ocean wave spectra. Breaking waves in the deep water section of up to 1.52 m high (5') can be generated as well as smaller waves. The wave frequencies range from about 0.25 cps to 1 cps. Several pre-cast concrete panels are available which are 3.66 m square (12'). These are used to help modify the water depth and to construct the beach section. Thus, various bottom configurations can be obtained.

## 3.2 False Floor and Beach Configurations

In order to maximize the wave induced bottom current, the false floor was constructed by the pre-cast concrete panels which were securely bolted to the wall 3.5 feet above the tank bottom and covering 120 feet length as shown in Fig. 3. In the end 96 feet section, a beach with 1/12 slope was constructed. The front end of the false floor and the tank floor were also connected by a 1/12 slope to form a smooth transition section. The gaps between the concrete

panels and walls were sealed with T-section and L-section steel members to prevent flow leaks through the false floor as shown in Figure 4. The test pipe was located 42 feet from the transition section and 78 feet from the beach. The reflection from this beach configuration is known to be small and less than about 5% for most wave periods.

# 3.3 Test Split Pipe

The split pipe is shown in Figure 1 which illustrates the bolting together of the upper and lower half sections and the mating of the assembled full pipe sections. The approximate inside and outside diameters of the middle portion of the pipe are 3.5 in. (ID) and 5.0 in. (OD), which does not include the bell ends or bolting flanges.

In order to evaluate the wave force coefficients, the displaced volume and the equivalent diameter of the split pipe are required. For this, the displaced volume of the test split pipe was measured as follows:

Two halves of split pipe were bolted together and openings at the sides and the ends were tightly shielded with masking tape. The displaced volume was measured by submerging the pipe in a 13.5" (ID) x 40 inch container and measuring the change in water surface elevations. Since the container was small, the volume of the split pipe was measured in two steps: first, the half including the outer bell end and then the half including the inner bell end. The volumes of each half and the inner bell ends only are:

 $\Psi_1$  = the displaced volume the half including an outer bell end = 474 in<sup>3</sup>

 $\Psi_2$  = the displaced volume the half including a middle portion = 832.0 in<sup>3</sup>

 $\Psi_3$  = the displaced volume of an inner bell end only = 161.0 in<sup>3</sup>.

As shown in Figure 5, the six halves of split pipe assembled together as a three jointed unit were used as the test section.

Thus, the total displaced volume, V, of the test section is:

 $\forall$  = the displaced volume of test section =  $3(\forall_1 + \forall_2 - \forall_3) + \forall_3$  $\forall$  = 3113.0 in<sup>3</sup>.

The "equivalent" pipe diameter D was defined by

$$V = \frac{\pi D^2}{4} 1$$

where 1 = the length of test section = 112 in. Thus, the value of D is given as:

$$D = (4V/\pi 1)^{\frac{1}{2}} = 5.95 \text{ in.}$$

The values  $V = 3113 \text{ in}^3$  and D = 5.95 in, were used to calculate the force coefficients throughout this report.

#### 3.4 Force Measurement Devices

The details of the test setup are illustrated in Figures 4 and 5. The test section is the assembly of a support pipe, six halves of split pipe, two force dynamometers, two shrouds and two support channels.

#### 3.4.1 Support Pipe

The support pipe is a 3.5 in. (OD) standard steel pipe with 3/4 in. thick flanges at both ends and is 114 in. long. The six halves of prototype split pipe were clamped onto the support pipe to form a solid piece of test section. Since the support pipe (tightly) fits in the split pipe, there was no chance of slippage. The flange angle of split pipe was changed by unbolting, reassembling and rebolting the split pipe in water without moving other units. The gap between the false bottom and the lowest points of split pipe was always 0.2 in.

### 3.4.2 Force Dynamometers

Two identical force dynamometers were located at both ends of the test split pipe. The close up view of a force dynamometer is shown in Figure 6. They were made of 1 in. thick ALCOA 6061-T651 aluminum plate. The 6 in. long sensing section is tapered to 0.6 in. square cross section. The strains in this section were measured by foil type strain gages to measure the wave forces in two directions. Four strain gages were used to form a bridge for each direction by a dynamometer, thus total of eight strain gages per a dynamometer. The force dynamometer has slotted holes at one end and a disk plate at the other end. The disk ends were connected firmly to the flanges of the support pipe and the slotted ends were firmly bolted to the support channels which were firmly mounted to the wave tank walls. Thus, the entire test split pipe section 114 in. long was rigidly suspended from both sides of the wave tank wall and nearly reached across the wave tank. In order to better approximate the two dimensional flow condition and to minimize the hydrodynamic forces on the force dynamometers, the force dynamometers were covered with shrouds made of 8 in. (OD) PVC pipe.

## 3.4.3 Strain Gage Signal Conditioners and Amplifier System

The strain gage outputs were amplified by a strain gage signal conditioner and amplifier system, Model 2100, Vishay Intertechnology, Inc., MacVern, PA 19355.

# 3.5 Wave Height Transducers

The water surface fluctuation at the test pipe was measured by a Sonic Profiler Model 86, Sonic Systems, Minneapolis, Minnesota.

#### 3.6 Visicorder

The horizonal and vertical forces from both the East and the West force dynamometers, together with the water surface fluctuation at the test pipe, were simultaneously recorded by a 6-channel Visicorder Model 1508, Honeywell, Denver, Colorado 80217.

## 3.7 Hot Film Anemometer

A hot film anemometer (Thermo Systems, Inc. system No. 1050-2C) was used to measure the horizontal velocity at 7 feet upstream from the pipe and at the elevation equal to the center line of the split pipe, i.e. 5 in. above the false floor. The velocity measurements were conducted to evaluate the force coefficients by using the measured wave kinematics and to compare them with the present results in the future (see Section 10.2). This data is not included in this report since the velocity measurements were outside the contracted work and were conducted at no expense to CEL. The data will be provided upon request.

#### 3.8 Propeller Current Meter

A propeller current meter (Model 401 & 403, Novar Electronics, Gloucester, England) was also used to measure the horizontal velocity for comparison with the hot film anemometer. The propeller meter was mounted on the opposite side of the tank wall to the hot film anemometer.

## 3.9 Magnetic Tape Analog Recorder

A 14-channel magnetic tape analog recorder (Bell Howell Model CPR4010) was used to simultaneously record the horizontal force, the vertical force, the water surface elevations at the split pipe and at the current meters, and the horizontal current for possible future analysis.

#### 4.0 DESCRIPTION OF EXPERIMENTS

### 4.1 Calibration of Force Dynamometers

The length of test split pipe, 112 in., was marked by nine stations, equally spaced, starting from the west end. At each station, the west and east dynamometers were calibrated in water by incremental loading and unloading of five lead bricks, each of which weighed 15 lbs., using very low friction ball bearing pulleys and cables. The calibration was made for the four directions, i.e. upward, downward, forward (North) (the direction of wave propagation) and backward (South). At the center of the pipe (station No. 5), the calibration was also made by using four 50 lb. lead bricks.

### 4.2 Torque Tests

Theoretically, an equally balanced four strain gage bridge should not sense a torque. The torque test was made by applying two 50 ft-lb incremented torque at the center of the pipe (station No. 5) using fixed bar and lead bricks.

### 4.3 Impulse Response Tests

In order to determine the natural frequencies of the test pipe-force dynamometer system, the impulse response tests were made by recording the force dynamometer signals during the free oscillation of the system induced by applying a certain load by hand at the center of the pipe and then suddenly releasing it. The tests were done in both horizontal and vertical directions.

# 4.4 Changing of Flange Angle

The orientation of the split pipe flanges was changed in water by two divers. A level bar and angle blocks were used to precisely set the flange at desired angles. The underwater photo of this procedure is shown in Figure 7.

## 4.5 Testing

For each of 168 runs, the wave and wave force signals were recorded by the visicorder. The wave, wave force and current signals were recorded by the 14-channel magnetic tape recorder. All recordings were made for the first 6 to 12 waves before the incident wave was contaminated by any possible reflected waves from the beach. Between runs, at least a five minute wait was allowed to make sure the water surface became calm before the next run. Example test waves are shown in Figures 8 and 9.

## 4.6 Changing of Water Depth

The water depth was changed by either adding or pumping out the water from the tank. To increase or decrease the water depth by one foot, it took about one hour. The water temperature varied from  $64^{\circ}F$  to  $70^{\circ}F$  during the experiments. The corresponding range of the kinematics viscosity  $\nu$  of the water is  $1.05 \times 10^{-5}$  to  $1.15 \times 10^{-5}$ . Since the variation is small, the average value of  $\nu$   $1.10 \times .0^{-5}$ , was used to calculate the Reynolds number throughout this investigation.

#### 5.0 ANALYSIS

#### 5.1 Horizontal Forces

The Morrison coefficient of drag and inertia will be determined from horizontal wave force data based on the Morrison equation and the Airy wave theory. The Morrison equation for the split pipe may be written as

$$f_{H}(\theta) = \rho V C_{I} \dot{u}(\theta) + \frac{1}{2}\rho DI C_{D}|u(\theta)| \cdot u(\theta)$$
 (1)

where  $f_{H}(\theta)$  = instantaneous value of horizontal force on split pipe at phase  $\theta$ 

 $\rho$  = density of water

 $\forall$  = displaced volume of split pipes = 3113 in<sup>3</sup>.

C<sub>I</sub> = inertia coefficient of split pipe

 $\dot{u}(\theta)$  = instantaneous value of horizontal acceleration of water particle at the center of split pipe

D = "equivalent" diameter of split pipe = 5.95 in.

1 = length of split pipe = 112 in.

Cn = drag coefficient

 $u(\theta)$  = instantaneous value of horizontal particle velocity at the center of split pipe.

# 5.1.1 Drag Coefficient

The value of  $C_D$  was evaluated at the wave crest and the wave trough from Equation (1) and the Airy wave theory, i.e. at  $\theta = 0^{\circ}$ ,  $180^{\circ}$  where  $\alpha = 0$ .

$$C_D = F_H(0, 180^0)/\frac{1}{2} \rho D1 U^2$$

where

 $F_{H}(o, 180^{O})$  = horizontal force at crest and trough  $U = maximum \ horizontal \ velocity from Airy wave theory given as$ 

$$U = \frac{\pi H}{T} \frac{\cosh k s}{\sinh k h} \tag{3}$$

where

H = wave height (measured)

T = wave period (measured)

 $k = 2\pi/L$ 

L = wave length

s = distance of the pipe axis from tank bottom

h = water depth

### 5.1.2 Inertia Coefficient

The value of  $C_I$  was evaluated at the wave zero-upcrossing from Equation (1) and the Airy wave theory, at  $\theta = \pm 90^{\circ}$ , where  $u(\theta) = 0$ ;

$$C_{I} = F_{H}(90^{\circ})/_{\circ} \Psi U$$
 (4)

where

 $F_{H}(\pm 90^{\circ})$  = horizontal force at zero cross

0
U = maximum horizontal acceleration from the Airy wave theory
and given as

$$U = \frac{2\pi^2 H}{T^2} \frac{\cosh k s}{\sinh k h}$$
 (5)

## 5.1.3 Maximum Force Coefficients

The maximum value of the wave forces are important in the design of pipelike structures. Sometimes the maximum horizontal force coefficient may be most simply defined as

$$F_{Hmax} = \frac{1}{2} \rho C_{Hmax} D \cdot 1 |U|U$$
 (6)

where  $C_{Hmax}$  = maximum horizontal force coefficient

F<sub>Hmax</sub> = maximum horizontal force.

### 5.2 Vertical Forces

The vertical water particle acceleration near the bottom is small. Thus, the vertical force component due to the vertical acceleration will be negligibly small compared to the vertical force component due to the horizontal velocity. The horizontal velocity induces, depending on the stage of wake formation, the downward force and the upward force. For the detailed discussion about this mechanism, the readers are referred to Refs. 4, 5, 6, 7, 8.

## 5.2.1 Lift Coefficient

The lift coefficients  $\mathbf{C}_{\mathsf{L}}$  will be evaluated for the maximum values of upward and downward forces as follows:

$$F_{v \text{ max}} = \frac{1}{2} \rho C_{l} D1 U^{2}$$
 (8)

where

F<sub>v max</sub> = maximum vertical forces.

Using the analog data recorded on photo-sensitive paper, for each and every run, the values of  $C_D$ ,  $C_I$ ,  $C_{Hmax}$  and  $C_L$  were determined together with the Reynolds number, Re = UD/ $\nu$  and the period parameter K = UT/D, where  $\nu$  = kinematic viscosity of water and D = "equivalent" diameter of split pipe.

## 6.0 CALIBRATION RESULTS

#### 6.1 Conversion Factors

The sample plots of the force dynamometers output reading in micro-strain ( $\mu\epsilon$ ) vs. the applied load in lbs. are shown in Figures 10 and 11. Both the west and east dynamometers show excellent linear responses. The values of the slopes, dR/dF, of the straight lines for all loading stations were determined. The distributions of dR/dF along the length of the test split pipe are plotted in Figures 12 to 15. These plots are similar to influence diagrams. The plots indicate that the response of the force dynamometers are practically equal for the upward and downward as well as for the forward (north) and the backward (south).

Assuming that the wave forces are uniformly distributed along the length of the split pipe, the conversion factors between the forces and the readings can be determined by calculating the areas under the influence curves.

The values of the conversion factor F/R, the ratio of the reading in micro-strain ( $\mu\epsilon$ ) to the total force on the test pipe in lbs, are tabulated in Table 4. Since the difference between the calibrations before and after the tests were small, the averages of the two values were used in the following wave force analysis.

The calibration signals equivalent to 80.7  $\mu\epsilon$  are always shown on the wave force records. Therefore, the signals are equal to:

#### West Dynamometer

calibration signal = 39.2 lbs for horizontal forces calibration signal = 34.4 lbs for vertical forces

#### East Dynamometer

calibration signal = 37.2 lbs for horizontal forces calibration signal = 38.2 lbs for vertical forces.

## 6.2 Natural Frequencies of Pipe Vibrations

The recording of the impulse response test is shown in Figure 16. Since the test split pipe portion was very rigid compared to the flexible dynamometers practically only the first mode of vibration exists. Thus the higher modes are negligible. It is shown that the first mode natural frequencies of the system are about 7.8 Hz in the horizontal direction and about 7.9 Hz in the vertical direction. They are one order of magnitude higher than wave frequencies. Thus the dynamic exitation of the pipe by wave should be small.

### 6.3 Torgue Test Results

The recording of the torque test is given in Figure 17. The undamaged west dynamometer showed a negligible response to torque while the east dynamometer showed a noticeable response to torque. Thus, the data from the west dynamometer may be more reliable than the data from the east dynamometer.

### 7.0 WAVE FORCE RESULTS

An example visicorder output for wave force tests is given in Fig. 18.

Generally, excellent data, similar to that shown in Fig. 18 were obtained for all 168 runs. The visicorder output of 168 runs and all the calibration data have been sent to CEL as a part of the August and September progress reports. They are not repeated in this report. The numerical values of various force coefficients, together with other wave parameters, are tabulated in Tables 5 to 18 as the computer printout for all 14 combinations of the water depths and flange angles.

The definition of the parameters are:

(The forces are in terms of total force on the entire test section 112 in. in 1bs.)

```
RUN
             = Run number
  H
             = Wave height in feet
  T
             = Wave period in sec.
FV+
             = Maximum upward force
FV-
             ≈ Maximum downward force
FHMAX
             = Maximum horizontal force in the direction of wave propagation
FHMIN
             = Maximum horizontal force in the opposite direction of wave
               propagation
FHC
             ≈ Horizontal force at crest
FHT
             = Horizontal force at trough
FH+
             ≈ Horizontal force at zero-up-cross
FH-
             = Horizontal force at zero-down-cross
CL+
             = Upward lift coefficient evaluated from FV+
CL-
             = Downward lift coefficient from FV-
CDMAX=CHMAX
             ≈ Maximum horizontal force coefficient from FHMAX
CDMIN=CHMIN = Maximum horizontal force coefficient from FHMIN
CDC
             = Drag coefficient from FHC
CDT
             = Drag coefficient from FHT
CI+
             = Inertia coefficient from FH+
CI-
             = Inertia coefficient from FH-
RE**5
             = Reynolds number in 105
             = Keulegan-Carpenter number
VEL
             = Maximum horizontal velocity in ft/sec from Airy theory
ACC
             = Maximum horizontal acceleration in ft/sec2 from Airy theory
WL
             = Wave length in ft.
```

## 7.1 Comparison Between the West and East Dynamometers

Example comparisons between the data from the west dynamometer and the data from the east dynamometer are shown in Fig. 19 for CDC vs. Re at  $\phi = 0^{\circ}$  and in Fig. 20 for CL + vs. Re at  $\phi = 0^{\circ}$ . For the cases shown, the agreements between the two sets of data are good. However, in order to avoid any possible contamination of the data due to the torque, only the west dynamometer data are used in the following analysis.

## 7.2 Comparison Between the Duplicated Tests

An example comparison between the original test series No. 9 ( $\phi$  = 0°, h = 8 ft.) and the repeated test series No. 11 is shown in Fig. 21 for CHMAX vs. K. The similar comparison between the series No. 4 and the series No. 7 is given in Fig. 22 for CL+ vs. K. For both cases, generally excellent agreements are shown. This is an indication that the data gathered are reliable.

## 7.3 Drag Coefficient

The plots of CDC vs. K with the Re and the water depth, h, as parameters are given in Figs. 23 through 26 for  $\phi$  = 0°, 45°, 90° and -45°, respectively. The similar plots of CDC vs. Re are given in Figs. 27 through 30.

For given values of  $\phi$ , Re and K, the values of CDC appear to be independent of the water depth, h. This is true for all other force coefficients. Ignoring the values of Re, the entire data of CDC are plotted versus K with  $\phi$  as a parameter in Fig. 31. The curves in the figure show the approximate envelopes of the data for each  $\phi$  values. The envelope values of CDC decrease slightly as K increases but are practically constant for larger values of K, say K >20.

The flange angle  $\phi$  dramatically influences the value of CDC. The envelope values of CDC for K >20 are 1.0, 3.0 and 5.0 for  $\phi$  = 0°, ±45° and 90°, respectively.

The CDC data with K >20 are plotted versus Re with  $\phi$  as a parameter in Fig. 32. The solid line in the figure indicates the CDC data for a smooth circular cylinder near a plane boundary obtained from the wave force tests (Re<10<sup>5</sup>) and the forced cylinder oscillation tests (Re >10<sup>5</sup>) given in Ref. 5. The smooth cylinder value of CDC decreases gradually from 3.0 at Re =  $10^4$  to 0.8 at Re =  $3 \times 10^5$  and then increases gradually to 1.1 at  $10^6$ . For the range of Re covered, the split pipe data show the similar tendency as the smooth pipe. When the flanges of the split pipe are parallel to the flow ( $\phi$  = 0°), the actual blockage area of split pipe is smaller than that of a circular cylinder with the same volume. As the flange angle  $\phi$  to the flow increases, the blockage area increases and becomes larger than that of the equivalent circular cylinder. The drag force increases as the blockage area increases. This tendency is clearly indicated by the data.

## 7.4 Inertia Coefficient

The plots of CI+ vs. K and CI+ vs. Re for the four flange angles are given in Figs. 33 to 40. For  $\phi$  = 0°, ±45°, the values of CI+ are nearly independent of and Re. For  $\phi$  = 90°, CI+ slightly increases as K and Re are increased. All of the CI+ data are plotted versus K in Fig. 41. The curves in the figure are the envelopes of the data. The value of CI+ increases significantly as the flange angle increases. This is also a blockage effect of the flanges.

The data of CI+ with K >20 are plotted versus Re in Fig. 42 and compared with the data for a smooth cylinder in Ref. 5. The comparison indicates that the split pipe with  $\phi$  = 0° has about the same or slightly smaller values of CI+ as a smooth pipe, and that the CI+ value of the split pipe with  $\phi$  =  $\pm 45^{\circ}$  and 90° are larger than the smooth pipe values

Virtually no difference was found between CI+ and CI- values as shown in Tables 5 to 18.

# 7.5 Maximum Horizontal Force Coefficient

The complete plots of CHMAX vs. K and CHMAX vs. Re are given in Figs 43 to 50.

Ignoring the values of Re, all the CHMAX data are plotted versus K in Fig. 51. In the figure, the solid lines indicate the envelopes of the data for different  $\phi$  values. The figure clearly shows that the maximum horizontal force on the split pipe drastically increases as the flange angle increases. The maximum horizontal forces for  $\phi=90^\circ$  and  $\phi=\pm45^\circ$  are respectively about three times and two times larger than that for  $\phi=0^\circ$ . This is a very important factor to be aware of for design of split pipe.

# 7.6 Lift Coefficients

The upward and downward lift coefficients CL+ and CL- are plotted vs. K and Re for each of the four flange angles in Figs. 52 to 67.

Generally, the upward lift coefficient CL+ increases from zero to the maximum and then gradually decreases as K is increased. However, the downward lift coefficient CL- monotonously decreases as K is increased. This is because the wake formation is small and the flow is more of a potential flow

situation for a small value of K. The flow through the small clearance between the pipe and the floor induces a large downward lift force as theoretically shown in Ref. 9. For a large value of K, the nonsymmetric shape of the wake creates a large uplift as clearly pointed out in Ref. 4, 5 and 7.

For the case of  $\phi$  = 0, vertical vibrations of the split pipe were often observed when the waves became large or large values of Re and K. A few points with extraordinarily large values of CL+ in Figs. 52, 56, and 70 are due to vibration and should be ignored.

In Fig. 68, the values of CL+ for all four flange angles are plotted vs. K. The solid lines in the figure are the envelopes of the data. The uplift force is greatly influenced by the flange angle. The force increases accordingly in the order of  $\phi$  = 0°, 90°, -45° and 45°. The uplift force on the pipe at  $\phi$  = 45° will be more than ten times as large as that at  $\phi$  = 0°. This is an important design factor to take into consideration.

The similar plots of CL- are given in Fig. 69. The downward lift force increases in the order of  $\phi$  = 90°, 0°, 45° and -45°. The downward lift forces have about the same magnitudes as the uplift forces. The lift force has at least twice the wave frequency. This may also be an important design factor to consider.

The lift force data for K >20 are compared with the data for a smooth circular cylinder in Figs. 70 and 71. Figure 70 indicates that the uplift force on the split pipe at  $\phi$  = 90° is about as large as that on the equivalent circular pipe. The split pipe at  $\phi$  = 0° has a slightly smaller value of CL+ than an equivalent circular pipe. The uplift coefficient for the split pipe at  $\phi$  = 45° is about four times larger than that of a smooth pipe.

## 7.7 Effect of Flange Angle on Force Coefficients

In order to summarize the effect of the flange angle on the wave force coefficients of the split pipe, the envelope values of various force coefficients at K = 25 are plotted versus the flange angle  $\phi$  in Fig. 72.

As can be seen, all of the wave forces are very strongly affected by the flange angle. All of the wave forces are minimum when the flange is parallel to the floor,  $\phi = 0$ . When the flange is perpendicular to the floor,  $\phi = 90^{\circ}$ , the horizontal forces are maximum and more than five times as large as the forces for  $\phi = 0^{\circ}$ ; but the vertical forces are minimum. The vertical forces are maximum and as much as five times the vertical forces for  $\phi = 0^{\circ}$  when the flange angle is  $\pm 45^{\circ}$  to the floor.

#### 8.0 ON THE APPLICATION OF THE PRESENT DATA

As demonstrated in Fig. 72, the single most important factor for the design of the split pipe is the flange angle. A slight misalignment of the flange from the horizontal position can increase the horizontal and vertical wave forces several times. Thus, very careful assessment must be made as to the range of the flange angle variation at the installation and the possible movements after the installation is in the field.

Once the design range of the flange angle is determined, the design wave forces may be determined as follows. The ranges of the Reynolds number, Re, and the Keulegan-Carpenter number, K, covered by the present tests are  $10^4$ <Re <2x10<sup>5</sup> and 0 < K < 40. Thus, if the design situations are within the range, the wave force coefficients determined from the tests can be directly used for design purposes. That is, if only the maximum horizontal and vertical forces are required for the design, they can be determined from the values of CHMAX, CL+ in Figs. 45 to 51 and 52 to 59 together with Eqs. 6 and 8. If the wave forces are required as functions of time, then the forces may be given by the Morrison equation, Eq. 1, and the drag and inertia coefficients determined from Figs. 23 to 32 and 33 to 42.

In most design wave situations, the Reynolds number Re becomes much larger. According to Ref. 5, the drag coefficient on a smooth pipe decreases from the subcritical value to a minimum value of 0.8 at about Re =  $3 \times 10^5$  and then seems to approach to a plateau value of 1.1 as Re is further increased as shown in Fig. 31. The split pipe data in Fig. 31 show the similar tendency for the Reynolds number covered. Thus, it may be reasonable to assume that the split pipe drag coefficients will also approach plateau values in the high Reynolds number range. Therefore, the drag coefficient given in Fig. 32

may be used for higher Reynolds number design situations. The same argument is true for the maximum force coefficients and the lift coefficient. Since the inertia forces are less important in the high Reynolds number design situations, the values given in Fig. 42 may be used.

## 9.0 SUMMARY AND CONCLUSIONS

The single most important design parameter for the wave force design of the split pipe is the flange angle  $\phi$ , the orientation of the split pipe flanges to the flow direction. Both the horizontal force and the vertical force are minimum when the flanges are parallel to the bottom. The horizontal force increases 3 to 6 times as the flange angle  $\phi$  increases from 0 to 90°. The vertical force increases up to 10 times as the flange angle varies from 0 to 45°. Thus, even a small misalignment of the flanges from the horizontal position can increase the wave forces several times and cause a pipe failure.

The drag, inertia and lift coefficients of the split pipe, obtained from the present tests, are correlated with the data of a smooth circular cylinder in Ref. 5. The trend of the data and the relative magnitudes of the force coefficients of the split pipe are found to be reasonable. This indicates the credibility of the data obtained.

The design criteria for the split pipe have been established which may be used even for the high Reynolds number design wave situations.

#### 10.0 SUGGESTED FUTURE WORK

This work focused on determining design coefficients for split pipe for the specialized case of wave forces on the pipe with the wave crests parallel to the pipeline. The force coefficients were determined by assuming Airy wave theory and utilizing periodic waves for a variety of water depths. Actual design and construction conditions can be considerably different from these special cases. Therefore, in order to significantly add to design information, particularly the determining of hydrodynamic forces on split pipe under actual environmental conditions, the following suggestions are made for future work.

## 10.1 Predicted Water Velocities vs. Theory

The velocities of the horizontal motion of the water at the level of the pipe were measured during the work described herein and it is important that a comparison be made between the water velocities measured and those predicted by the Airy wave theory. During the testing, these data were recorded on 16-channel magnetic analog tape. This can be reproduced onto a visicorder paper trace recording or it can be digitized and processed digitally. Thus, the water motion at the pipe level experienced in the Wave Research Facility can be compared with predicted water motions in the ocean and an estimate can be made as to the validity of the Airy theory used and the resulting predictions of wave forces on split pipe.

# 10.2 Mean Square Error Method for Determining Coefficients

In another research project at QSU, the comparison of using the maximum value method for determining drag and added mass coefficients vs. using the

minimum mean square error method has been made at QSU. It was found that it is possible for the coefficients to be somewhat higher for the minimum mean square error method of evaluation rather than from the maximum value method. However, it is anticipated that less than a 10 to 20% difference will occur. This should be evaluated in order to determine if any change could be significant for design for split pipe in waves as utilized by the Navy. The computer program for determining the wave force coefficients by the least square method is available at Oregon State University but it will require some revisions for this work. This can be done with the data from the tests that are described in this report.

### 10.3 Skewed Waves

In nature the waves approach the pipelines in directions which are seldom such that the wave crests are parallel to the pipe alignment. More likely the waves will be oriented with the wave crest perpendicular to the pipe alignment or at some other angle of skewness. The effect of the skewness angle on such lift and drag coefficients should be investigated as a fairly high priority activity. Such tests are particularly difficult to perform and would probably have to be accomplished with long sections of pipe mounted differently than for this report and perhaps in a shoaling condition rather than for a horizontal bottom. Careful end conditions must be provided so that the leading end does not unnecessarily influence the data obtained. It is possible for a steady state uplift to occur from waves when the wave crests are perpendicular to the pipe center line.

## 10.4 Combined Effect of Current and Waves

It is particularly difficult to model current and waves superimposed in a laboratory. However, such a condition is common in nature. One possibility for investigating at least some aspects of this phenomenon and how it affects hydrodynamic loading is to tow a pipe section near the bottom into the waves and in the same direction as the waves to get an approximate idea of these combined forces. Powerful towing equipment does exist at OSU for towing such a pipe specimen. Thus, the combined effects of current and waves can be investigated at least approximately by towing the split pipe sections spanning the 12 foot width of the wave tank with waves.

# 10.5 Alternative Split Pipe With No Flanges

When the flanges of the split pipe are oriented at some angle to the incident flow, the wave forces can be increased up to six times, as found in this report. This may cause failures of pipe lines composed of split pipe. In order to eliminate this undesirable affect of split pipe flanges, a new design is suggested -- a split pipe without flanges. Given that the split pipe was purely cylindrical sections, much data already exist from various researchers as to the forces from waves and current. However, it is unlikely a new design will result in a purely cylindrical shape. Therefore, the shape will have some irregularities in order to accommodate a bolting arrangement. Any irregularity from the cylindrical shape will lekely influence the lift and drag coefficients for design purposes. Therefore, it would be very desirable to test such designs in the Wave Research Facility to obtain comparisons with the work accomplished in this report. Given that additional testing to determine the effects of skewness and the other items

which appear in this section on the standard split pipe with flanges, then it is likely that additional testing for other shapes will not need to be so comprehensive. Fewer tests may be needed in order to determine comparisons between other shapes and the split pipe flange sections as used herein.

## 10.6 Random Waves

The Wave Research Facility at OSU has a capability to produce wave spectra that closely approximate wave spectra in the ocean at a scale ratio of 1:10 or better. It would be useful to the Navy to investigate the influence on the hydrodynamic coefficients on split pipe due to irregular waves vs. periodic waves. This work can be done for various water depths as in this report.

### 11.0 REFERENCES

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Table 1 - TESTING SCHEDULE AND MODIFICATIONS

Day #	Date	Work (Scheduled)	Work (Done)
1	Mon., July 18, 1977	Floor construction	Floor construction
2	19	Test pipe installation	Floor construction
3	20	Test pipe installation	Test pipe installation
4	21	Calibration in air	Test pipe installation (east force dynamometer bent)
5	Fri., July 22, 1977	Calibration in water	Examination in air
6	Mon., July 25	Test series 9, series D	Calibration in water
7	26	8, dewater	Series 1, Series 2
8	27	4, series 5	series 3, watering, series 4
9	28	6, dewater	series 5, series 6, series 7
10	Fri., July 29	1, series 2	watering, series 8
11	Mon., Aug. 1	Test series 3, watering	Series 9, series 10, series 11
12	2	7, "	Series 12, dewatering, series 13
13	3	11, "	Dewatering, series 14
14	4	Calibration in water	Calibration in water
15	Fri., Aug. 5	Clean up	clean up

Table 2 - TEST COMBINATIONS (series number)

Wt. Depth	4 feet	6 feet	8 feet
00	1	6	9,11
45 <sup>0</sup>	2	5	10
90°	3	4,7	8
-45 <sup>0</sup>	14	13	12

Table 3 - VALUES OF CLEAN MAXIMUM WAVE HEIGHTS (feet)

Wave Period Depth	2 <sup>sec</sup>	4 <sup>sec</sup>	6 <sup>s3c</sup>
4 ft	1.7	2.1	2.1
6 ft	2.0	3.0	2.8
8 ft	1.9	4.0	3.3

Table 4 - Conversion Factors of Force Dynamometers

		a. Area (με ft/lb)	b. Conversion factor (1b/μ¢)	c. Calibration Pulse (1bs)
	1	22.7	.410	<u>.</u>
West	2	21.2	.441	
Vertical	3	22.0	.426	34.4
West	1	19.5	.478	F F F F F F F F F F F F F F F F F F F
Horizontal	2	18.9	.493	<b></b>
nor rzonca i	3	19.2	.486	39.2
East	1	21.1	.442	
Vertical	2	18.6	.503	
	3	19.9	.473	38.2
East	1	20.7	.461	
Horizontal	2	20.3	.461	
nor izoneai	3	20.5	.461	37.2

a. = Area below the influence curve

b. = 1/a = conversion of reading in  $\mu\epsilon$  to total force in lbs.

c. = Equivalent total force in lbs. of calibration pulse of 80.7  $\mu\epsilon$ 

<sup>1. =</sup> before tests

<sup>2. =</sup> after tests

<sup>3. =</sup> average of 1 and 2

Table 5 - Wave Force Tests Data and Calculated Results for Series 1; h = 4 ft,  $\phi$  = 0°

1	•	Ĭ	•	9	-	1	211	.			1	A	ON CO	NA.	FUR	BES:	r QU	TO.	DD,C	PRA	ĊTI.	CAR	4		1
*	20.620	15.333	9.834	6.450	5.641	4.332	2.947	1.953	31.323	21.374	12.959	7.573			-										
RE ** 5	1.152	.456	695.	.360	.635	. 684	.329	.218	1,241	.1%	.483	.282	M -F T	43.049	43.049	63.049	43.049	19.087	19.087	14.087	16.047	66.517	66.517	66.517	2: 5:5
-13	694.2	2.427	291.2	1.970	2.398	2.577	2.976	1.769	3.768	3.637	5.999	1.342	-13	2.257	2.203	2.290	1.746	2.313	55 4. 5	2.654	1:922	3.548	3.688	2.607	***
1:	. 320	.201	1.146	.376	.281	151.	2.142	.163	164.	676.	1.477	1.374	•10	2.121 2	.083	.749	1.752	.141	.155	2.293	.824	.576	:593	.260	
COT	.216 2	.312 2	1 829	.504 1	1.016 2	.474 2	2 300 0	. 000 2	1.276 1	1.356 1	2.214 1	1 027.	cot	2 515	315 2	1 991.	356 1	. 357 2	. 323 2	. 300 2	1 600.0	.10i i	1 912.	. 395 1	
363	. 169.	.781	. 622.	. 504	-	. 669.		.000	.319 1.	.607 1.	.233 2.	.339	200	. 627 .	670	. 518	. 162	. 159.	. 649.	.003 6.	.000 0.	256 1.	537 1.	975 1.	
			-		12.1 5		6 0.003	0,			-	-					•				0		131 .5		
VINOS >	1.172	1.651	4 2.305	3.025	2 4.165	1 5.871	3 9.964	.9.110.433	1.196	2.403	3.44.	7 1.743	K C041N	191.2	3.005	. 4.85	669.5	9 6.036	92211.188	15717.774	19.619	2.15	1 6.3	1 5.45	
COMAX	1.40	1.941	2.84	1.152	3.962	165.5	7.173		.71.	1.137	1.587	3.097	COMAX	2.510	3.260	4.764	5.712	7.439	4.42		3.49219.07619.431	1.127	1: 31	2.729	
2	.641	1.016	1.927	1.149	1.481	1.528	2.407	1.253	.194	1.247	1.857	1.500	-13	1.062	1.853	3.625	2.681	2.963	2.972	3.0581	3.492	.452	3.214	3.203	
3	.593	110.	1.603	0.000	. 463	.388	.628	0.000	. 295	.740	646.	.380	5	.617	.766	3.554	00000	1.270	1.127	2.243	2.090	1.091	1.875	509.4	
1	36	5.32 1	14.47	4.68	37.07	30.34	23.87	9.46	.94	23.51	11.75 1	3.07	=	1.67	2 66.5	.33	7.66	35.76 1	28.95 1	2 62.12	0.22 2	15.76 1	28.94 1	19.22 4	
į	.55 34	22.97 25	13.02 1	89.8	35.26 3	24.34 30	17.18 2.	11.39	18.08 37	10.95 2	. 79 1	.55		29.76 11	21.74 22	11.70 15	69.2	13.10 3	25.41 2	18.39 2	70 1	*	8.16 Z	5.12 10	
FHT	6.33 32	5.16 22	4.52 13	9 57.	4	.52 26	21 09.	0	1.46 18	60	.39 5	. 27 .	FH	.30 29	.11 21		1.62 7	51 33	.26 25	00	.6 3.	.21 44	.63	5 :20	
	.25 6.	1		45 1.	AG 9.6	•	01 0.	13 0.0	.,	18.	=	-		•	47 5.	C.	8: 1.	45 8.	34 4.	30 0.	30 9.	59 17.	-	-	
CHS	20.5	12.65	9.14	-	10.	3.62		6.03	10.45	9.51	6.33	2.35	¥	18. 19	10.4	5.45	•	5.		0.1		*	1.32	5.32	
FHAIN	34. 16	25.77	15.37	8.63	37.97	30. 34	23.47	11.33	69.67	33.64	17.77	3.07	FHHTN	53. 15	44.70	12. 36	16.35	71.52	97.10	42.57	23.4.	33.75	ñu . 52	3 6.5	
FH4A3 FH41W	41.23 34. 16	98.6	18.90 15.37	9.64	35.26	24.94	17.14	9.40	24.41	15.91		5.43	FHYEY	73.57	52. A3	11.77	16.39	66.21	50.03	36.74	20.06	45.14	54.92	14.0.	
		4.00 16.30 16.47 29.84 25.77	12.85	3.29	13.16	1.91	5.77	1.32	65.9	17.46	9.55 8.6ª	5.64	: .	31.13	30.03	24.17	69.2	26.37	15.39	7.32	3.06	15.30	50.54	16.44	
	*** 17.52 14.74	. 36 1	6.49 1	6 33	4.10 1	2.91	1.5	.0.0	10.03	10.36 1	16.03	.67		67.79 3	44.84 3	53.73 2	0.0	11.33 2	5.83 1	5.47	2.13	37.1# 1	26.25 4	23.76 1	
TOT VA	17 00	10		6.63	1						16 16		DINAMONETER ACC FV+	4.01 47											
EST 0		. 4 .			4 2.00	5 2.00	5 2.00	5 2.00	3 6.00	00.4	90.00	9 6.00	ts.		1 2.99	16.1 5	92.1 0	29.1	3.37	3 2.29	1.52	5 2.84	1.85	1.12	
QUN H-FT 1-3EC FVE	1 2.00	69.1 3	56.	.63	1.64	1.25	.45	.55	2.03	1.39	.79	94.		2.50	1.30	1.22		11	1.07	.73	•	2.75	1.77	1.07	
5	-	2	-	•	5	•	•	•	•	13	=	12	2	•		•	•	5	•	1	•	•	13	=	

2 300		¥	189.8	216.5	2,042	11.420	160.02	13.958	9.728	5.604	33.323	51.879	12.622	7,573	RRC	IS P	AGE	FUR	BES	T Qi	JALI	I TY	PRA	dri	CAR	12	
		RE 5	.635	. 325	.228	.636	1.122	.780	.543	.313	1.241	.615	.470	.282	MFT	19.087	14.087	18.087	63.0.89	43.049	43.049	63.00	43.049	115:99	66.517	215.99	66.517
	13	-13	2.748	2.967	2.180	3.956	2.156	2.285	2.868	2.134	1.974	1.203	1.184	2.211	-12	2.643	2.965	1.900	3.944	5.040	2.761	2.851	1.920	1:757	.92k	146.	1.459
	3333FEE	•10	1 2.531	254.2	8 2.018	5 5.352	3.214	3.427	2.416	3,414	5 5.276	999.9 9	3 3.379	3 3.000	.10	3 2.325	115.5 1	1.435	6 4.518	86 W. 2 4	1 2.416	155.2	1.754	22015	9 4.346	3.767	3.144
	FIPE 9.	coc cor	.3.2.438	.934 1.547	1.573 1.573	526 . 355	325 N	73539	1.192 1.109 2.614	96 . 835	505. 69	417 .986	97 1.148	58 1.543	COC COF	92 2.583	30 1.457	5. 1.481	96 1.325	34 .367	159. 19	.9 1.044	1.571	36 .320	13 .153	11 1.012	
	4		15 2.743	-		22 5,752	45 1.634	16 2.073		54 2.088	.585 2.349	2	182 3.297	181 2.058		84 2.292	04 1.430	364 1.454	A.484 5.206	70 1.394	155 1.664	33 .948	74 1.544	41 1.936	97 5.143	51 2.877	616 1 237
	LENGTH	COMAX COMIN	.080 4.815	9.67010.056	9.75410.540	.240 4.022	133 1.345	3.25A 1.616	4.213 2.910	013 3.75	. 551 .5	501 1.119	5.196 1.4	3.910 2.861	COMAX COMIN	4.716 9.184	.02120.104	74516.3	1	003 2.570	\$76 3.955	7.738 5.533	637 6.174	717 1.041	tat 1.997	247 2.651	
	F 13	כר- כט	.535 5.	4.862 9.	2.580 9.	5.862.7.	1.953 2.	2.453 3.	3.787 4.	3.651 6.	.5 81 2.	1.011 3.	2.025 5.	2.531 3.	פו- פו	250.	9.71317.	3.1A617.	.18212.	3.60512.003	4.199 5.	5.296 7.	7.27210.637	1.292 4.	.44H 5.	3.601 1.	
	1.8017	•10	4 116.2	1.646 4	5 757.		1.916.1	4.803 2	3.459 3	2.933 3	3.930	4.854 1	6.436 2	2.663 2	:	6 120 9	3.895 9	0.000 3	.6523.67510.1 4212.867	7.794 3	9.227 4	1.263 5	6.733 7	1 950.	9.548 1	0 *** *	
	¥0104E=	Ŧ	42.50	23.51	12.12	30.7412.157	29.48	21.70	18.99	1.14	19.89	7.96	4.52	5.06	#	40.87	23.50	19.56	30	29.64	28.22	19.07	7.32	17.71	6.11	3.751	36
	TOTAL V	į	69.00	19.22	11.21	41.59	43.95	32.55	18.63	13.02	53.17	30.74	15.19	1 6.87	FF	15.95	1 19.30	10.20	35.11	19.63	26.75	16.93	F.59	31.15	38.76	16.34	
2366	E	HC FHT	41 21.70	52. 3.62	16.1 16	72 8.32	40.6 2	15 7.23	1. 7.23	52 1.61	3 19.89	5 14.47	10 5.61	62 2.71	HC FHT	40 22.99	36 3.4	7 1.70	1 11.92	79 10.22	98.6	9 6.81	3. 3.41	4 17.71	\$ 12.00	4.94	
	~		26.		-	51.	3 45.5	27.	1.7.	3	6.67	1 35.4	16.	ř		0	3.	1.67	9 46.9	38.	\$ 22.43	6.1		\$ 55.4	111.4	. 14.34	
11100011	ite= 45	FH4I	42.96	23.5	12.12	16.17	37.4	21.70	14.99	9.14	19.8	16.78	7.23	5.96	FH4AX FH4IN	41.74	67.00	21.12	76.29 46	71.52	53.15	36.10	17.71	35. 42	29.29	12.34	
	FLANGE ANGLE + 5.00=0	FV- FH4AX FH4IN	45.21	22.61	12.11 76.5	55.10	65.10	+3.76	27.49	13.02	16.81	51.36	25.32	6.87		17.50	9.11 22.71 19.79	20.46	115.70	134.06	74.90	\$0.49	21.74	43.33 166.51	93.63	45.14	
			40.36	11.37	2.97	52.72	54.36	32.95	24.71	7.91	19.77	14.83	9.9	45	- 7.3	10.57	22.71	9.06 3.06	91.56	102.55	26.40	61.32	15.75	43.3	21.2.	17.54	
5	. OFE T	WEST DINAMON.T. P	2.00 25.91 40.36	.84 2.00 3.64 11.37 22.61 23.51 4		4.00 109.31 52.72 55.10	4.00 168.9A 54.36 65.10 37.43 45	*** 32.95	4.00 22.56 24.71 27.49	4.00 6.35 7.91 13.02	2.03 6.00 133.77 19.77 16.81 19.89 79	1.33 6.00 71.20 14.63 51.30	.77 6.00 31.42 9.51 25.32	69	EAST DINAMONET'F	53.59	9.11		2.22 212.89 91.5¢ 115.70	3.91 216.90 102.55	2.72 123.94	1.89 47.19 61.32	1.07 14.54	253.72	1.69 139.94	1.09 65.42	
*2-61 MOV • 200 6317-54	WATER DEPTH= 6.0FE T	ZUN M-FT T-SEC FV		2.00	10.0						6.00	6.00	6.00	6.00	IST DIN	4.42	2.27	1.59						4.98			77
25.4163	-	3UN H-FT	13 1.64		16 .59	17 1.11	17, 1.95	14 1.35	19 .94	20 .54	21 2.03		3	94. 45	RUN VEL	13 1.41	13 .72	16. 81	24.1 7	1 2.43	14 1.73	13.1 61	69. (	1 2.75	18.1 2	21 1.0¢	
	3	: 2	-	. 15	-	1	-	-	1	2	2	22	39	-	: \$	-	-	-	11	11	-		2	21	22	~	36

	4.00 10.35 4.00 10.39 6.00 7.19 6.00 35.16
13.45       50.54       96.97       90.75       20.06       17.03       42.63       37.46       6.204       6.0206.50415.360       3.415         13.45       24.57       50.56       97.60       97.20       6.17       28.42       28.10       4.91910.14421.37321.164       3.265         2.55       6.59       25.08       27.25       1.67       3.41       12.54       13.62       2.220       5.73521.41723.702       1.654         102.54       9.52       25.08       27.25       1.67       3.41       12.54       13.62       2.220       5.73521.41723.702       1.654         30.30       2.93       43.64       29.76       7.06       16.19       4.962       .4691       7.920       3.921       7.306         30.30       2.93       43.64       29.76       7.06       16.19       4.962       .4691       7.821       7.826         32.54       13.62       25.31       13.61       23.27       1.001       23.27       1.666       3.27       1.667       3.67       3.67       3.67       3.617       3.336       3.375       1.667       3.375       3.477       3.336       1.667       3.577       3.576       3.477       3.336	42 6.00 11. 45 6.00 10. EAST OINAHOR. FEL ACC FV
102.94 9.52 264.17 130.73 110.35 41.89 55.17 35.08 3.082 .285 7.920 3.921 3.308 30.39 2.93 33.64 47.65 7.06 16.39 16.19 4.962 .46913.397 7.635 4.766 32.39 13.92 94.30 57.90 23.91 9.88 20.57 20.26 4.431 1.68012.736 7.920 3.223 14.76 14.65 75.11 27.25 8.19 3.62 12.04 13.29 .965 .970 2.322 1.803 3.523 10.612 25.64 33.63 20.44 111.75 27.25 55.51 23.84 2.199 .703 2.567 .569 3.612 51.04 16.11 210.77 51.39 41.93 25.54 31.77 15.33 3.375 1.6615.257 3.375 5.417	
10.58 14.65 15.11 27.25 0.19 2.62 12.04 13.29 .965 .970 2.324 1.80\$ .542 00.22 25.64 43.63 20.44 111.75 27.25 55.51 23.84 2.199 .703 2.567 .560 3.612 51.04 16.11 210.77 51.39 41.93 25.54 31.77 15.33 3.375 1.66515.257 3.375 5.417	4.28 102. 1.85 30. 2.42 32.
23.70 7.12 134.33 27.13 16.29 12.77 15.05 17.36 4.266 1.31919.742 5.329 6.512 2	2.97 60. 1.92 5i. 1.16 23.

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s Data and talculated Results for Series 4; h = 6 ft, o = 90°
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Table 8 - Wave Force Tests
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Table 9 - Wave Force Tests Data: and Galculated Results for Series 5; h=6 ft,  $\phi=45^{\circ}$ 

	7	3.943	2.947	2.011	1.344	23.591	16.242	9.908	160.9	36.454	24.124	14.072	8.376													-
	RE 5	624.	.334	.225	.150	1.316	.907	.553	.340	1.358	.898	1524	.312	M-FT	19.623	19.623	19.623	19.623	51.295	\$1.295	51.295	51.295	80.515	40.515	A0.515	40.434
	-10	2.420	5.659	2.973	2.076	1.806	2.127	156.2	2.400	1.509	1.239	1.572	1.999	-13	2.523	2.618	2.737	2.001	2.210	2.157	2.778	594.5	134.1	164.	1.400	5 0 17
9. 3333FEE	•15	2.472	2.447	2.543	1.730	2.934	5.945	2.951	2.400	3.937	3.470	3.187	1.785	11:	155.5	2.262	2.504	1.828	126.5	2.974	2.728	2.017	3.540	1.375	3.064	
	COT	1.021	1.323	3,242	1.815	.473	. 194	1.069	1.414	. 355	.604	1.043	1.682	100	196.	1.384	2.299	1.709	.534	. 469	1.006	1.664	.335	.342	. 194	
OF PIPE	565	2.974	2.205	1.945	1.815	2.081	2.297	2.325	1.764	2.665	1.194	2.980	1.430	202	2.669	2.038	2.544	1.674	2.06%	\$.2.5	2.223	1.307	2.464	1.649	3.336	
LENGTH OF	COMAX CONT	186.91	1 9.554	14.599	15.243	1.549	1 2.585	3.206	3.889	.622	1.257	. 2.444	2.355	COMÁX COMÍN	14.390	57918.269	726.864	84229.390	1.603	1 5.617	8 A.052	. 8.655	1.233	1.524	1 4.499	
	1	. 6.841	8 8.451	3.98712.96814.589	4.29412.70215.243	3 2.648	8 2.983	3 4.409	284.737	5 2.407	. 3.19	9 3.934	5 4.037		9.17013.544	.12116.57	7.22225.47726.864	026.84	2 2.536	9 6.251	6 9.993	4.4.4	3 1.437	. 3.375	1.541	
1.9017 FT3	-13 •	7 4.64.	9 4.218			9 1.413	969.2 5	3 4.503	1 4.702	507. 0	.92¢	6 2.579	4 3.065			710.12		0 7.35026.	298. 5	5 5.839	4 4.766	5.70110.309	1 2.213	A 1.69.	1 6.941	
	F. C.	13 2.667	10 1.359	. 1.94	50 2.348	3.75	1 4.595	19 5.013	120.8 3	3,390	14.640	9 6.776	16 3.494	FH- CL.	.0 5.816	.29 2.66710	99 2.287	12 0.000	2 1.735	44 7.415	7310.234		152.7 75	800.5 64	5.9613.396	
VOLUME =	FH: F	36 25.32	19 21.70	.7 16.2A	33 7.60	16.82 20	55 23.51	19 19.89	36 4.95	.0 16.64	10.6 4	69.9	52 5.06	FH. 51	75 26.40	39 21.	14.	54 7.32	31 35.42	23.	in 118.	\$6 10.22	11 15.67	93 5.79		
TOTAL	FHT	4.15 25.86	3.26 19.49	3.62 14.47	.90 6.33	1.03 .7.02	7.23 32.55	7.23 19.49	3.62 9.95	47 43.40	45 25.32	6.33 13.56	3.62 4.52	FHT	.92 26.75	3.41 18.	55 13.71	.85 6.69	44 .6.91	9.51 31.77	6.41 16.	4.26 A.	62 46.11	5.81 16.33	5.45 13.04	
EE	9	12 4.	53	11	. 06	57 14.	59 7.	71 7.	52 3.	51 14.47	79 10.85	.9 40	10	# C#	87	02 3.	.2 .0	,,0	32 23.44	8. 8.	9.0	34 4.	32 13.62	19 5.	90	
	2	13 12.	11 5.	.5 .	. 09	66.76 79.	47.02 41.	21.73 15.	15 6.	25. 12 104.	.95 56	18.	16 3.	=	. 10.	K, 5.	17 2.	. 50	11 78.	.1.	.9 15.	.8. 3.	11.72 130.	.6 10.	.62 50	
61 E= 45	FHM TN	3 24.33	13.51	1 16.29	3 7.50	1 . 66 . 7			6 4.95		. 22.	23.87 14.87 18.	90.5	FHMIN	1 54.54	3 44.95	2 29.97	14.65	1 61. 1	105.1	64.49	£ 22.14		\$ 27.75	\$ 21.75	
FLANGE ANGLE: 45.90EGR	FHAMA	28.0	20.8	4.45 14.47	6.3	101.2	54.5	29.84	15.1	114.2	56.7		9.6	FH4AA	55.17	40.83	24.42	13.39	16.9	113.6	60.19	26.37 26.66	25.65 01.06	61.99	42.12 51.83	
1		16.	10.1	:	2.14	54.03	52.12	33.93 30.44	12.03	16.47	16.47	15.65	6.54	: 2	23.76 37.36	24.93	9.04	3.66	66.35 32.96 16.97	106.21	59.31			30.21		
S. OFEET	WEST DING MONETCE	2.00 10.46 16.47 28.03	2.00 3.34 10.38 20.80	2.17	2.00 1.17 2.14 6.33	4.00 143.75 54.03 101.27	4.00 43.57 52.72 54.25		00 7.96 12.03 12.12	6.00 138.06 16.47 114.29	6.00 82.74 16.47 56.79 22.42 56.	6.00 41.12 15.65	6.00 7.52 6.59 8.68	EAST DINAMON TER ****	23.76	6.56	2.5	0.00	56.35	3.16 134.9A 106.21 113.69 102.19	69.20	1.19 14.54	1.15 295.24	89.31	62.23	
	1-3ec			2.00				4.00		6.00				T OINA	2.99	2.33	1.57	1.05	*.59	3.16	1.93			5.09	1.22	
MATER DEPTHE	34-7	1.9	1.54	1.0	.6	16.5 15	5.05	35. 1.25	11. 88	5.79	54 1.85	59 1.05	+9. 19		. 35	.7.	.50		26.5	2.01	1.23	.75	1.91	1.99	1.16	
1	3	63	9	51	25	15	3.6	35	35	25	5.8	- 59	5 42	¥0×	3	53	15	25	53	5	53	55	25	5	5	

IS BEST QUALITY I 14.742. 6.969 161. 23.454 3.784 2.848 1.991 23.551 16.242 9.908 35.516 6.091 .222 .873 .549 . 423 106. .553 .340 .107 2.525 1.515 ' 1.323 .089 1.316 .260 19.623 51.295 51.295 19.623 19.623 46.515 AD :515 COT CI+ CI- WL-FT 1.81 1.81 3.62 4.52 2.022 2.214 3.038 3.038 1.215 1.215 1.716 2.145 . 215 2.550 1.198 7 6.51 7.23 0.000 9.41437.20541.339 0.000 0.000 3.003 3.336 7 3.41 22.57 25.54 1.570 2.78311.43412.940 .635 .863 2.192 2.480 1.70 16.72 19.56 2.376 3.18514.54217.033 .727 .741 2.127 2.492 0.00 il.37 13.24 1.667 6.69720.79124.290 0.000 0.000 2.098 2.451 0.00 5.02 6.81 0.00020.93057.32877.453 0.000 0.000 2.313 3.142 . 865 . 5 03 2.232 2.400 6.61 30.93 26.95 1.525 1.992 2.186 1.514 .612 .178 1.930 1.806 .401 1:403 1.616 .241 2.116 7.24D 0.00 13.56 13.56 0.000 1.80A12.40212.402 0.000 0.000 2.502 2.502 .396 2.291 2.373 .534 2.414 2.548 7.23 3.62 10.85 6.33 1.882 1.138 2.172 1.383 1.086 .543 2.434 1.420 7.19 7.15 1.425 2.291 5.622 5.592 0.000 0.000 1.735 1.726 .176 2.491 1.332 2.21 16.33 5.46 3.632 5.439 4.01? 2.144 1.034 ..332 2.256 1.337 20.40 1.81 1.81 10.99 20.40 .363 1.576 9.258 9.044 .786 .746 2.416 2.646 .189 2.144 2.031 .707 0.000 2.007 2.007 -13 3.62 22.61 26.22 0.000 1.460 5.726 6.642 1.145 .916 2.195 2.546 9. 3333FEET 6.41 2.379 2.499 1.575 1.414 .942 -101 2.543 Table 10 - Wave Force Tests Data and Calculated Results for Series 6; h=6 ft,  $\phi=0^{\rm o}$ COT LENGTH OF PIPE= .842 5.11 23.41 25.20 2.305 2.900 3.052 2.771 .552 3.41 4.40 2 4.424 6.740 4.577 1.123. 14.99 8.62 18.04 8.50 .942 .410 1.770 .730 1.127 . 905 COC 3.62 16.24 17.18 1.679 2.191 2.672 2.592 1.149 FHT FHE FH- CL+ CL- COMAX COMEN COC .474 1.182 .851 .662 . 646 CL+ CL- COMAX COMIN 15.35 16.14 3.770 4.329 4.941 4.751 .316 7.23 27.13 16.24 .605 .239 1.170 .543 .906 1.790 1.442 .327 1.266 3.252 3.252 24.75 14.30 1.320 1.421 2.163 TOTAL VOLUME= 1.6017 FT3 32.55 25.32 7.23 34.36 32.55 .809 7.23 25.32 26.22 1.332 FH4IN FHC FHT FH+ FH-0.00 8.32 8.32 1.70 18.19 4.14 DATA 7/29/77 . 0145P4-0300PM 7.23 0.00 0.00 3.41 0.00 6.81 1.19 2.51 FLANGE ANGLE = 0.006GREE 4.52 1.67 0.00 0.00 11.75 7.79 32.55 FHC 1.91 0.03 31.10 23.41 10.01 5.45 0.00 15.84 69.9 1.07 8.32 20.38 12, 30 26.22 N. 6A 4.52 13.52 26.67 13.56 MINH 51.99 39.17 57.90 50.41 32.36 21.94 26.22 17.54 35. 42 14.33 14. 17 6.41 FV- FH4AY 10.99 65.14 7.32 33.4. 10.03 3.67 18.90 FH-4AX 22.74 5.77 22.61 18.12 45.21 45.21 29.84 4.52 93.60 33.66 13.56 32.55 14.61 19.00 8.32 14.47 55.51 14.34 93.6 6.51 51.47 26.75 10.03 .... 7.58 9.23 6.92 7.32 3.66 24.24 16.47 3.29 1.9. 0.00 1.65 3.29 .... 76.18 54.20 29.30 24.94 62.12 6.53 5.86 36.67 - 14 . SERIES NO.6 . PUN 61-72 WEST DINA MONETER 0.00 . . . . . 0.00 EAST DINAMONETER 30.92 23.40 6.20 11.37 3.61 5.47 1.82 0.0 40.10 . 94 15.46 15.54 25.52 3.65 58.33 41.92 8.54 51.0. 25.52 6.0FE.T RUN H-FT T-SEC FV+ 4.00 65 2.97 4.00 00.4 9.00 6.00 2.95 1.95 2.00 1.49 2.00 2.00 2.00 1.25 4.00 2.00 6.00 2.25 1.55 .62 4.59 3.16 1.93 1.19 3.07 2.03 1.24 99. WATER DEPTH 14. .11 1.79 71 1.13 .53 1.01 2.95 21.2 69 .6. 64. 1.23 2.91 .75 1.94 1.22 .20 26.2 2.01 .53 \*\*\*\* 29 63 2 . 19 99 9 22 3 59 99 69 63 6.9 63

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Table 11 - Wave Force Tests Data
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	*	3.784	3.923	2.031	1.274	22.739	16.648	9.989	6.091	37.526	24.124	14.742	4.577												
	5 * * 3 d	.423	.427	.227	.142	1.270	.930	.558	.340	1.397	96 9*	645.	.319	W-FT	19.623	19.623	19.623	19.623	\$1.295	\$1.295	\$1.295	51.295	90.515	80.515	40.515
_	-13	3.424	2.607	3.304	2.659	2.805	3.033	3.459	3.054	1.593	1.943	950.1	2.789	-10	3.472	5:454	3.236	2.553	2.641	3.006	3.257	3.247	1.801	1.867	1.819
3333FEET	120	3.512	2.433	3.271	2.607	102.4	3.767	3.459	2.182	4.143	3,166	950.4	160.2	:15	3.247	2.410	2.473	2.410	3.890	3.247	256.5	211.2	4.125	3.567	1.750
•	COT	3.664	169.2	054.4	4.0 37	1.915	1.136	2.234	1.768	204.	1.217	2.444	1.604	500	2.504	2.745	4.799	3.801	1.146	1.194	2.475	1.332	74	. 155	2.557
or plots	263	4.80ª	2.915	6.357	4.037	1.551	3.407	3.680	2.121	3.101	794.4	5.431	2.005	263	4.235	2.613	5.143	3.732	3.039	2.800	3.402	1.634	2.712	1.750	5.021
LENGTH 0	COMIN	10. 106	7.177	250.91	695.0	2.131	3.596	5.967	6.575	1.073	2.02#	3.802	4.171	COMIN	90319.641	4.192	12.925		1.057	5.989	10.644	10	. 477	.96.	6.155
2	COMAX	.67310.76410.306	7.625	4.34317.48316.052	3.67723.18520.589	4.565	5.300	7.229	6.717	*.107	3.492	7.603	6.414	COMAX	13.9031	6.54014.51414.192	.87532.32632.925	9.81137.32339.535	2.266	.20113.324	.22414.33910.644	4.29611.76613.11	3.475	2.836	13.557
17 FT3	3	4.673	2.778	4.3431	3.677	.139	.129	.359	2.254	0.000	.277	.297	.877	ct.	11.31713.		12.875	9.8113	.514	.2011	1.2241	4.296.1	.339	2.260	1.78713.
1.4017	: 3	161.,	2.073	1.763	0.000	1.453	1.776	2.673	1.960	.930	1.500	2.058	1.854	מרי	6.92511	3.526	1.92212	0.000	2.968	1.248	4.875	5.273	. 363	2.967	1.434
くりしじゃきゃ	ŧ	35.26	27.13	19.27	9.22	43.40	34.35	23.51	12.66	19.08	14.47	19.08	7.23	ŧ	15.76	15.52	17.84	8.85	40.87	14.06	22.14	13.62	20.44	13.62	17.03
TOTAL V	ŧ	16.17	25.32	18.09	9.34	65.10	45.68	23.51	9.04	47.02	58.94	18.08	5.43	į	37.44	25.08	15.68	A. 36	61.09	36.78	20.06	8.16	46.41	26.75	16.72
	. 1	14.47	10.65	5.06	1.41	36.17	21.76	15.17	4.52	17.36	21.70	16.28	3.62	FHT	11.67	11.07	5.45	1.70	\$0.A7	28.22	17.03	3.41	20.44	17.63	17.63
JEGREE	FHC	18.99	11.75	7.23	1.81	126.59	65.19	25.32	5.43	133.43	19.61	36.17	4.52	FHC	16.72	10.53	5.85	1.67	198.34	53.50	23.41	4.13	117.04	66.99	13.44
. 30.3	FINE	69.99	24.9. 11.	13.27	4. 22	4.04 162.76 75.15 126.	64.72	41.95	16.42	46. 30 133.	36.17	25.12	9.40	THM IN	74.33	57.22	37.46	16.72 17.71	37.57 198.	3.85 137.29 114.44 53.	73.23	34.06	.2.23 117.	17.12	.6. 77 51.
FLANGE ANGLER 90. 13EGR	MINHS XVINS	45.50	30.74	4.94 19.89	9.04	52.76	12.10	49.73	17.14	17.23	69.90	99.66	15.37	FH4AX FHMIN	18.59	58.52	36.78	16.72	90.78	17.29	38.65	10.10	37.26	50.92	10.29
FL ANGE	::	18.45	11.20 30.74	46.4	1.69	4.04.1	2.47 101.27	2.47	5.77	0.00 177.23	4.34 104.89	1.00	1.99	: .	66.51 78.53		14.65	4.39	14.32	3.85 1	8.42	16.97 10.16	14.63 167.26	.0.31	11.96
-1			1.36 1	2.01	0.03	51.82	33.93	18.39	5.01	40.12	26.7.	13.71	4.10		27. 34 6	14.22 26.37	2.19 1	0.00		24.61	33.5.	13.49 1	15.54	52.96 +	12.21
6.0FE	INAMON E	2.00 16.55	2.00 A	2.00 2	2.00 0	4.00 51	4.00 33	4.00 16	4.00 5	6.00 40	6.00 26	6.00 13	* 00.9	DINAHOH ACC F		2.94 14	1.58 2	. 66.	4.43 105.81	3.24 24	1.94 33	1.19 13	1.25 15	2.09 52	1.28 12
WATER DEPTH= 6.0FEET	SUN H-FT T-SEC FV+				.65 2.	2.97 4.	2.10 4.	1.26 4.	., 11.		1.85 6.	1.13 6.	.9 99.	EAST DINAMOMETER	.94 2.95	.95 2.	.50 1.	.32	2.42 4.	2.06 3.	1.24 1.	.15 1.	3.10 3.	1.99 2.	1.22 1.
TE?	: z	71 1.95	74. 1.97	75 1.05	. 92	77 2.	78 2.	79 1.		91 2.	82 1.	13 1.			73	7.	. 21	. 92	77 2.	79 2.	79 1.	. 64	11 3.	1 24	41 1.

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e Tests Data and Calculated REsults for Series 8; $h = 8$ ft, $\phi = 90^{\circ}$
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Table 12 - Wave F
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PAGE 8		<b>y</b>	2.001	1.556	1.112	.667	158.82	19.125	12.530	35.645	25.461	14.937	7.695	*	RO	C	OPY.	FUR	NIS	HED	10	D,C	-	+	+
		86 ** 5	.224	.174	.124	.075	1.666	1.068	.700	1.327	946.	.556	.287	W-FT	20.202	20 - 2 0 2	202.02	20.202	57.584	\$7.584	57.584	11.871	11.17	11.871	149.10
	ET	-13	3.653	3.642	3.526	2.490	3.864	2.779	3.1.62	2.013	1.409	204.2	3.100	-13	3.439	3.355 3.417	3.095 3.377	2.533	5.323 3.291	4.240 2.74A	2.745 3.198	4.963 2.211	112.5	292:2	126.5
	9. 3333FEET	CDT C1.	67 3.487	08 3.202	626.2 00	00 2 4 90	13 4.935	169.8 90	74 3.394	.743 5.368	.918 6.106 1.409	52 3.203	90 2.331	•10 100	56 3.223			0.000 2.487 2.533				. 875 6.963	. 157 4.777 2.211	96. 2.961	46 2.474
	±3014	0 000	4.095 3.767 3.407 3.653	2.709 2.709 3.202 3.842	0.000 0.000 2.929 3.526	0.000 0.000	2.512 1.413 4.935	2.439 2.008 3.891 2.779	3.007 3.174 3.394 3.182	2.601 .7	1	3.703 1.052 3.203 2.402	2.990 2.990	9 000	3.463 3.856	2,503 2,550	0.000 0.000	0.030 0.0	2.177 1.331	2.25. 1.644	2.471 2.532	2.405 .4	3.035	3.911 1.494 2.961 2.262	3.645 2.346 2.474 2.927
	LENGTH OF	COMIN		24.369 2			2.433	3.012	4.511		.166 5.025 2.148 3.459	4.338 3	5.980	CDMAX COMIN					1.072	166.9	7.466	.656			
	FT3 L	CL- COMAK COMIN	7017.199	2620.307	0126.004	8536.853	.179 4.082	.261 4.590	52.5.680	12 4.459	520.5 99	.2 89 6.348 4.338	716.9 06	CL- CDMAX	1531.800	6742.555	9749.063	5473.595	966 1 69	2. 9.150	.33610.194	2.069 2.190	3.154 1.594 1.023	2.44612.716 7.771	3.62411.26910.447
	1.6017 F	3	1514 5.6	.251 5.4	0.000 5.80126.00431.310	0.00012.00536.0536.053	1. 676.	1	27.13 1.930 .152 5.680	.756 .212 4.459 1.338	. 757 .1	2. 794	7.23 2.303 1.090 6.977	٠ تا •	3,30210.61531.60033.931	5.45610.96742.55543.343	10.2210.69712.89749.06359.966	0.00029.85473.59574.954	6.484 1.669 1.996	1.479 2.324 9.150	1.313 .3	6.367 2.0	1.469 3.1	1.573 2.9	3.615 3.L
		¥.	510.818.01.7101.514 5.67017.19818.017.	13.56 16.20 1.251 5.42620,30724,369	8.96 10.67 0	4.52	54.25	36.17 1.326		21.70	47.02 10.05	32 12.66 14.47 10.85 1.467		#	18.73	14.47		4.60	57.90	35.76	27.25	23.84	17.03	10.22	6.41
	TOTAL VOLUME=	FHT FH+	66 18 99	1.61 13.56		25.4 91	10 86.81	19.05 19	36 26.94	18.72 46		56 14.47	.3 5.43	FHT FH	17.56	12.41 04	1.00 9.36	16.4.61	31 33.63	11.55 11	55 23.41	6 53.50	13 36.76	12.14	6.63
1000AM-	386	FHC	25	16	00.00 00.00	0.00 0.00	5.74 65.10	19.05 64.1	34.36	1.27 28.94	1.72 16.00		5.43 5.43	FHC	3.85 4.26	1.67 1.70	0.00.0	0.00 0.00	1.37 61.31	15.55 42.57	5.75 30.65	1.63 34.66	1.14 17.03	5.75 10.22	6.63 4.26
. 111621	99.00	1	.4 66 .61	16.29	10.67 0.	4.52	12.12 11	15.95 61	.8. 93 .32	52.09 13	.2.69 68	19.65	36.01	HAIN		20.95			9. 19 100	\$6.02 5	15.15 26.	15.54	20.44 6	51.13 26.	11.75
0ATA 7/29/77	FLANGE ANGLE 99.00EGR	FV- FH4A, FH4IN	6.25 16.99 19.99	13.56_	4.86	4.52	190.001	6.59 115.74 75.95 61.	1.65 61.49 44.43 32.	173.61	99.83	43.60	12.65	FH4AY FH4IN	35.11	29.62	16.72	9.03	91.96	58.63 230.73 126.02 56.	3.66 110.35 A5.15	15.27	62.26 31.77 20.44 60.	96.96	5.49 24.0.
96-	FLAN		6.25	3.62	1.94	1.69	4.24		1.65	4.24	3.29	1.99	1.96		11.72	3.65 7.32	4.39	3.66	16.91	58.63		15.08		26.14	
SERIES NO.8 . RUN AS-36	1. OFEFT	DINAMONETER	2.00 1.67	2,00 3.52 13.56 16.29 1.	87 1.03 2.00 0.00 1.94 4.85 16.67	2.00 0.00 1.49 4.52 4.52 0.	4.00 45.13 9.24 198.09 112.12 115.	4.00 33.43	4.00 20.09	93 3.23 6.00 29.42 4.24 173.61 52.09 131.	94 2.31 6.00 15.04 3.29 99.83 42.69 68.	99 1.35 6.00 10.03 1.99 43.40 29.65 25.	5.00 4.18 1.90 12.65 10.95 5.	EAST DINAHOMETER	.50 1.56 3.65 11.72 35.11 37.46	1	.87 3.65 4.39 16.72 20.64	.52 0.00 3.66 9.03 9.20	3.20 5.03 296.92 76.91 91.96 49.79 100.	3.72 47.39	16.22	1.09 247.99 80.57 15.27 25.54 93.	2.20 29.16	1.29 10.75 26.14 86.94	.67 6.56
. 8.0N	WATER DEPTH= 8. OFEFT	PON H-FT T-SEC FV+		16. 2.00	13 2.00					3 6.00	11 6.00	15 6.00	0. 6.00	EAST DINA	1.56	.39 1.21	18. 15.	.17 .52	0 5.03		15 2.44			1 1.29	
SERIES	1754 DE	4-H NO.	98 1.85	96.1.60	1.0	1862	99 1.02	16.2 66	91 1.95	3.5	16 2.3	15. 1.3	96 .70	<b>3 4 6</b>	e. St	. 9	570	1. 6	3.5	16 2.37	H 1.55	11 2.95	is 2.19	95 i.23	3.
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Table 13 - Wave Force Tests Data and Calculated Results for Series 9; h  $\approx$  8 ft,  $\phi$  =  $0^{\rm O}$ 

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	*	950.2	1.556	1.056	.750	25.060	261.52	18.465	12.200	5.403	35.080	24.669	14.711	7.355									1					
	S ** 3 d	.230	.174	.118	.084	1.400	1.407	1.031	.681	.324	1.306	.919	.548	.274	W -FT	202.02	202.02	20 - 202	20.202	57.584	57.584	57.584	57.584	57.584	41.671	41.471	149.10	
-	-13	2.585	2.562	168.5	1.948	121.5	2.110	2.101	2.396	2.061	1.705	1.697	1.829 >	1.789	-13	2.436	2.211	2.311	1.584	1.598	1.391	2.114	150.2	2.070	1.926	1.233	1.5.11	
9. 33 38 FEET	•13	2.585	2.348	2.454	1.771	1.697	1.546	2.015	1.361	2.198	1.705	2.424	5.445	2.032	•10	5.539	5.289	2.327	1.537	1961	1.951	5.129	2.417	1.949	161.1	1.2.5	2.631	
	001	0.000	0.000	0.000	0.000	.418	.331	. 308	.352	.349	.384	. 398	.273	.545	100	6.000	000.0	0.000	0.000	00000	951.	.145	. 132	.367	0.00	001.0	0.000	
Se Fibe	200	0.000	0.000	0.000	0.000	.545	.744	. 309	. 352	.383	. 575	.776	. 545	.545	202	0.000	0.000	0.000	0.000	.618	.611	.427	.326	. 363	.355	.117	. 59.	
ENGTH 0	COMIN	12.405	16.245	145	\$29.624	.435	.476	1.123	1,939	3.505	.480	. 679	1.227	2.400	COMIN	10	28.045	183	11.677	.553	169.	1.130	3.313	7.043	245.	.380	2.054	
-	COMAX	41212.40512	2.46614.49216.	. 60622.93322	0.00023.29525.624	1.036	1.240	1.262	1.586	3.739	1.055	1.358	2.182	2.727	CL- COMAX	14024.37023.36	6.63229.037	43.49143.	43.07342	1.091	1.035	1.209	3.910	6.524	1.064	2.311	4.296	
17 FT3	2	=	,	0		.548	.376	.617	1.669	2.128	.262	.371	.869	2.186	-5			5.95443	0.0004	.508	.586	.428	5.353	4.259	36	3.02	4.524	
1.4017		0.000	1.251	0.000	0.000	4.015	3.620	.853	176.	1.080	4.167	.359	1.004	1.009	5	0.000	2.729	0.000	0.000	4.043	3.832	.194	2.842	1.570	4.667	1.174	2.191	
VOLUME=	Ŧ	14.47	10.45	6.87	3.98	36.17	36.17	26.40	19.89	8.14	18.04	12.66	8.14	3.98	i	13.62	9.37	6.64	3.24	27.25	23.44	26.57	17.03	1.17	20.44	9.29	6.41	
TOTAL V	i.	14.47	9.95	7.05	3.62	28.94	28.94	25.32	16.28	8.68	18.03	18.33	12.56	4.52	Ŧ	14.21	9.70	69.9	3. 34	13.44	33.44	26.75	20.16	7.59	21.15	16.72	11.70	
•	TH.	3.00	00.0	9.00	0.00	18.08	14.67	7.23	3.62	96.	14.47	7.23	1.81	06.	FE	0.00	0.00	00.0	0.03	0.67	6.81	3.41	3.41	. 85	9.6	3.5	0.03	
9.13EGREE	FHC	0.00	0.00	0.01	0.00	25.32	12.55	7.23	3.62	. 90	21.70	14.47	3.62	.90	FHC	0.00	0.00	0.03	0.00	26.75	26.75	10.01	3.34	š.	13.34	13.14	3.34	
	FHHTN	14.47	10.45	6.47	3.98	36.17	38. 14	26.40	19.49	8.14	18.04	12.66	8.14	3.98	FHHT	27.25	18.73	13.24	6.47	27.25	26.37	26.57	34.96	16. 15	2,.44	18. 43	13.52	
FLANGE ANGLES	FHYAX	14.47	3.95	7.05	3.62	64.85	54.25	29.66	16.20	9.68	39.79	25.32	14.41	4.52	FHIAL	29.42	19.39	13.38	6.69	46.81	46.15	28.45	.0.13	15.30	13	43.41	24.42	
FLANG	: :	1.65	1.65	00.0	0.00	23.72	14.47	14.50 29.66	17.13	46.4	9.38	6.12	5.77	3.62	: .	1.66	4.91 19.39	1.43	00.0	24.15	19.52	10.01	36.46	9.49	14.31	56	19.03	
133	FVE	0.00	. 96.	0.00	9.00			20.06 1	10.03	15.51	7.12	69.9	69.9	1.67	FV+	0.00	1.62	9.00	0.00			4.56	29.16 5	3.65		21.41 5	14.54	
WATER DEPTH: 8.0FEET	H-FT T-SEC FVE	2.00	2.00	2.00	2.00	4.00 173.83	1.00 167.15	2 00.4	1 00.4	00.4	5.00 157.12	6.00	6.00	00.9	EAST DINAMONETLE	1.60	1.21	.82	.54	4.84 176.80	4.90 167.69	1.59	2.39 2	11.11	3.0% 167.69	2.13 2	1.27	
JEP 14:	153H	1.90	1.44	.97	69.	1.90	3.92	2.47 4	1.90	, 06.	3.18 6	2.24 6	1.37 6	.67	EAST VEL	.51	.39	•26	.19	1.11	3.12 4	2.29	1.51	.72	2.90 3	2.00.2	1.22.1	
WATER	SUN .	16		66	101	101	101	192 2	103	101	105 3	106 2	101	101		46		6	103	101	101	2 201	193 1	10.	105 2	106 2	107 1	

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Table 16 - Wave Force Tests Data and Calculated Results for Series 12; h=8 ft,  $\phi=-45^{\rm O}$ 

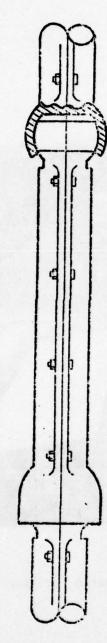
	2	•156	1,556	.112	.711	24.730	599	5.605	002	35.645	568	277	.355		180	M CK	Y	UR	NIS	HED	10	TY F	-	-	1
		2.	7	1.	•	24.	14.465	5.	12.200	35.	24.895	15.277	7.												
	RE ** 5	.230	.174	124	.079	1.381	1.031	.313	.681	1.327	.927	.569	.274	WFT	20.202	202.02	202.02	20.202	57.584	\$1.584	\$7.584	57.584	118.16	149.16	41.871
	-10	11872	202	5.869	148	2.364	2.735	5.845	8 32	2.013	922	.349	439	-13	96 9	118	250	6.38	922	304	232	199	969	608	115.5
	910	.908.2	0.000 0.000 2.369 3.202		5.987 5.90125.91229.799 0.000 0.000 1.568 2.148				21.51 2.606 4.334 3.606 2.890 1.934 1.057 2.176 2.632		.619 3.122 1.922	~	2.439 2.439	•10	1.434 1.460 2.688 2.690	0.000 0.000 2.566 2.614	0.550 0.650 2.487 2.426	0.000 2.159 2.638	.598 2.186 2.228	2.129 2.304	2.411 2.232	2.215 2.461	.437 2.481 1.896	.629 2.665 1.809	16 2.
		~	2.3	2.510	1.5	2.364	2.303	2.371	2.1	2.684	3.1	6 5 2 3 4 9			2.6	2.5	2.4	1.2	1.5	1.5.1		2.2	3.5	1 2.6	.95.F. F. 589
1	COT	1,551	0.000	0.000	0.00	. 806	.923	2.087	1.057	.557	. 619	1.264	2.192 1.091	COT	1.460	0.00	0.000	0.00	.59	:623	1.572	. 564	.43	.62	.95.
	000	1.551	000	9.000	000	1.372	1.306	2.087	934	1.486	1.809	5.529	182	202	434	900	000	0.000	1.427	1.265	2.315	1.629	1.460	1.843	2.104
	Z				0 66		62 1.	26.2.	90 1	. 973 1.	95 1			Z									.1001.		
	COM	13.4	120.3	155.4	2.62	1.5	1.6	5	2.8	1	1.2	2.023	3.273	100	1.12	35.6	45.4	73.2	1.0	3.167	9.433	5.310		1.97	1. 116
	COMAK COMIN	6.35613.95513.490	6.90614.95120.307	7.73522.29925.473	5.91	2.500 3.002 1.544	3.30	4.679	3.80	2.787	2.190 1.295	3.540	4.363	COMAX COMIN	5.80	2.545	4.151	9.89	3.172 1.050	6.885	9.570	7.169	2.934	7.395	6.427
	-13	3561	9061	7352	9012	200	794	235	3.84	2.116				7	1892	3533	1214	1195		7.073		9.179	1.223	4.127	9.8 13
	•13				87.5		2.631 3.462 3.308 1.662	4.784 6.235 4.675 5.426	90	.687 2.	14.47 1.461 3.157	1.870 4.146	5.41 3.327 4.968	5	16.1512.50213.16925.60427.742	11.9213.64515.35332.54235.694	7.3211.76716.12144.15742.476	5.1126.11613.11959.69273.201	2.118 1.433	64 7.	8.75012.172				
		8 4.299	\$ 4.505	8.68 4.905		39.79 1.923			1 2.6	- 1	1.6	1.6	1 3.3		112.5	13.6	111.7	1.921	5 2.1	. 2.884		3.552	2.107	3.186	1.568
	Ŧ	15.73	13.56	8.6	4.16	39.7	34.36	10.05	21.5	21.70	16.4	10.85	5.4	Ŧ	16.1	11.9	7.3	5.1	37.46	26.82	9.51	20.44	20.44	13.62	10.33
	#	16.24	12.66	7.60	3.62	39.79	28.94	9.04		28.94	23.51	10,85	5.43	#	15.05	10.47	7.32	4.18	36.78	26.75	9.20	18.39	26.75	20.95	A. 16
	FH	1.81 10	1 00.0	0.00	00.0			4.52	1 50	0		9.04 1	1.81	FHT	1.70	0.00	00.0	0.00			3.41	6.81 18			
						34.00	. 21.70		19.09 10.05 10.00	7.12	34.36 15.55		1					1	1 25.20	2 14.65			5 17.63	111.92	, A. A.
	FEC	1. 11	0.00	0.00	0.00	57.87	10.74	4.52	19.8	57.97	34.3	18.04	3.62	FHC	1.67	0.0	6.63	0.00	60.13	28.42	5.05	16.72	56.95	15.11	16.46
	MIN	.73	95.	8.68	.16	01.	90.	.75	99	00.	54.59	19.	. 43	MIN	.36	. 34	.65	10.22	44.28	74.93	39.	64.46	52.72	37.46	72. 4.1
	i v	15	£_13		2	65	39	3 11	67 9	1 34		2 14	3 5	T. X	32	4 23	5 14				3 20				
	FV- FHYAK FHMIN	7.41 16.29 15.73	12.6	7.60	3.6	26.5	11.1	10.1	39.0	08.5	41.5	25.3	7.2	NIHA XAKHA -VA	34.1	21.74 23.44	15.05 14.65	8.36	33.7	61.8	20.7	73.5	10.3	40.4	
		.41	.61	19.	.82	1 143	.39	.51	640	37.1	96	.65	200	1	.38			1.83	. 4.1	1 12.	.37	35	61 1	1 12	13
			1	~		105	=	13	66	95	99	52		2	15	9.11 10.25	4.01 5.49	1	9	166	56	100	4.7	12	7.0
	FV	5.01	3.01	1.67 2.64		76.89	61.84	10.36	26.74	26.74	27.75	13.37	5.52	FY	14.58	9.11	4.01	3.65	19.31	67.80 166.27 161.85	18.96	36.45 100.35 73.57	A2.02 47.61 110.35	68.51 79. 20 140.44	75.5
	RUN H-FT T-SEG FV+	00	134 1.44 2.00 3.01 4.61 12.66 13.56		2.00 .84 .85 3.62 4.16	4.00 76.69 105.43 126.59 65.10	4.00 61.84 61.39 77.76 39.06	.47 4.09 10.36 13.51 10.13 11.75	140 1.90 4.00 26.74 44.45 39.06 29.66	1.23 6.00 26.74 62.37 104.51 34.00	142 2.26 6.00 27.75 59.96 41.59	A 143 1.39 6.00 13.37 29.65 25.32 14.47	144 .67 6.00 5.52 8.24 7.23 5.43	EAST DINGHONETER ****	1.60 14.56 15.30 30.10 32.36	1.21	.87	.55 3.65	4.61 89.31 60.47 133.76	3.59	1.09 10.96 26.37 20.73 20.44	2.36	3.05	2.15	161 1.36 1.19 26.5. 70.19 68.89
	1-1-3	133 1.90 2.00	. 2.	135 1.03 2.00				7		3 6.	9 9	.9	.9 2	AST	.51 1.	9 1.		. 18		3 3.	9 1.				
	: 1	1.9	1.4	1.0	99.	137 3.85	138 2.47		1.9	1.2.	2.2	1.3	9.			.39	.23		137 3.07	5.29	69.	1.51	1+1 2.95	5.06	1.26
	3 C.	133	134	135	136	137	138	139	140	141	142	143	144	3	133	136	135	136	137	134	130	14.9	1.1	145	

Table 17 - Wave Force Tests Data and Calculated Results for Series 13; h = 6 ft,  $\phi$  = -450

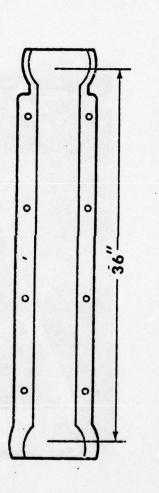
-	Ì	6	Ì	•	9	°	1	. 1	1	TROM	S PA	GR PY	IS I	EST Q	JAL	İTY	PR	CT:	CA,	BIL	1		-			•
	¥	3.595	2.898	1.892	1.294	21.927	15.836	10.151	4.873.	34.846	22.784	13.402	6.701	WASHEL	10	DDC	-	-		1						
	8 * * 3 a	007	.323	.211	.145	1.225	.685	.567	.272	1.298	976.	664.	.250	# - F T	19.623	19.623	19.623	19.623	51.295	\$1.295	51.295	51.295	915.09	40.515	90.515	40.515
ET	-10	996.2	2.876	2.610	2.053	1.697	2.853	5.749	2.727	1.373	2.362	5.499	2.677	-13	2.618	5.600	2.611	1.933	1.594	2.528	2.588	2.568	1.293	1.977	2.101	2.353
3333FE	•13	2.873	2.761	2.810	2.310	4.121	2.450	2.356	2.454	3.432	3.149	2.677	5.499		2.742		2.760	2.135	3.610	2.482	2.420	5.269	3.406	2.912	2.475	2.062
FIPE= 9.	C C01	5 1.531	9 2.359	2 1.832	000.0	9999	3 1.046	9 1.273	9 1.657	6 .389	.750	1.643	1.971		164.	-	1.725	000.00	6 .616	769	\$ .959	3 1.560	1 .234		619.	1 1 . 4 45
10	N COC	992. 9	1 2.359	3 1.632	5 0.000	9 1.855	3 2.713	2 1.909	. 2.20	2 1.516	2 2.723	1.943	3 2.629		7 .472	4 1.818	969°I 0	3 0.000	0 1.416	1 2.514	2 2.353	\$ 2.043	A 1.439	5 2.237	4 1.645	1 2.735
LENGTH	AX COMIN	10 8.166	34 9.431	4.65716.12	17.61215.655	92 1.30	193 1.883	63 3.18	28 5.52	_	33 1.592	257 2.629	14 3.94	AX COMIN	9914.41	.45117.774	3629.33	6529.48	60 2.26	22 3.821	251.5 10	1.23510.40	854. ES	0+0 2.155	36 3.094	16 6.930
F 73	CL- CDMAX	509 7.910	448 9.439	84114.6	56517.6	2.982 3.492	4.192 4.1	942 3.563	541 6.62	302 1.111	.453 4.093	3.5 986	16.5 747	L- COMAX	95315.09914.41	74217.4	98728.73629.330	51132.56529	147 6.960	104 4.122	191 4.001	5.66111.2	374 3.325	217 9:0	3.30413.935	7. 14. 112. 771. 11. 916
. 8017	כר •	.9 600	338 6.	234 5.	809 3.	.765 2.9	2.030 4.1	587 3.	063 6.	.680 2.	113 4.1	429 5.	3.644 5.747		6.69713.9	.24412.1	.77312.	. 889 9.5	409 6.147	.6 6 9.	4.722 9.021	6.23515.6	.196 1.	.463 6.2	9741	611.31
-	±	28.94 4.	22.61 3.	4.47 4.	7.23 1.	25.32 1.	10.74 2.	18.99 4.	9.04 3.	14.4711.	16.28 4.	10.13 2.	5.43 3.	ź	.54	20.44 9.	14.4714.	6.81 7.	23.84 3.	27.25 4.		4.51 6.	13.62 .	18.62 2.	8.51 3.	:
TOTAL VOLUME	. I	28.03 2	21.70 2	14.47 1	A.14	61.49 2	26.40 7	16.28 1	8.14	36.17 1	21.70 1	10.95 1	5.06	ŧ	26.75 25	20.06 2	14.21	7.52	56.45 2	2 51.92	16.72 17	7.52	46.13 1	20.06 1	10.01	
10	1	5.43	5.43	1.81	0.09	21.76	14.08	9.64	2.71	14.47	11.94	90.6	2.71	E	1.70	3.41	1.70	00.0	20.44	13.62	6.81	5.55	4.86	6.81	3.41	
JE GRES	FHC	2.71	5.43	1.41	0.00	61.49	47.02	13.56	1.62	36.42	43.40	21.70	3.62	FHC	1.67	;;	1.67	0.00	60.13	44.17	15.72	3.3.	53.59	15.11	29.95	
FLANGE ANGLE =- 45. )	FHHTN	26.74	22.61	16.41	7.23	43.40	32,55	22.61	9.04	36. 19	25.12	14.47	5.43	FHYIN	51.09	40.47	58.95	11.62	74.33	20.99	40.47	17.03	17.11	37.45	17.03	
SE ANGL	FH4Ax	28.03	21.70	14.47	9.14	115.74	75.95	25.32	10.95	115.74	65.19	16.82	4.14	FH4AX	53.50	.0.13	24.82	15.05	30.73	79.00	56.85	14.39	21.85	45.79	61.19	
		23.06	14.83	5.11	1.65	98.44 115.74	72.44 75.95	24.01	10.71	45.66 115.74	70.84	35.95	16.7		77.64	24.30	12.42	4.39	05.10	43.84 157.4" 140.44	60. 99	19.52	51.27 175.72	94.46 145.79	73.25	
FECT	TUN H-FT T-SEC FV+	14.21	9.19	4.14	.84	58.50	35.10	32.59	5.01		10.20	13.37	5.01	OMETER FV+	23.70	96.41	14 56	3.65	4.27 113.01 205.10 230.73	43.84 1	16.19	10.21	7.20	32.81	71.17	
MATER DEPTH= 6.0FE'T	T-SEC	2.00	2.00	2.00	2.00	00.4	00	00	*.00		6.00	6.00	6.00	EAST DINAMOMETER	5.79	52.2	1.47	1.01	1 12.4	3.04	1.94	.95	3.02	1.97	1.16	
14 OEPT	H-FT	1.85	1.43	16.	.67	2.17	2.00	1.24	.62		1.74	1.03	.51	-		.72	14.	.32	2.72	1.36	1.75	.60	2.89	::	===	
HSTE	NO.	145	146	147	144	149	151	151	152	153	154	155	156	3	163	145	147	13.8	149	150	151	152	153	15.	135	166

6.345 1.193 21.360 .986 15.861 1.285 34.501 .222 .501 .349 .579 .821 .516 .354 45.61 13.62 26.75 27.59 1.931 8.446 5.169 3.161 2.641 .785 2.478 2.556 43.849 .920 2.039 2.270 43.049 74.93 73.57 11.92 30.46 37.46 2.086 6.207 5.315 2.382 2.339 .379 2.646 2.578 45.049 15. 35 4. 64 2. 55 10.03 7. 66 3.478 3.501 5.193 5.522 1.647 .970 2.324 1.775 43.849 41.61 17.43 33.44 27.25 .600 6.426 1.513 .373 2.567 .467 3.205 2.611 .66.517 43.59 1.67 1.70 21.74 21.80 6.77012.92215.14516.191 .621 .632 2.556 2.563 18.087 36.85 11.92 10.10 11.62 1.86010.573 4.756 1.429 1.817 .800 4.514 21843 -86.517 71.09 120.86 16.94 93.44 21.74 16.18 43.47 41.72 7.98613.579 9.768 9.375 2.442 1.818 2.911 2.698 18.087 65. 37 10. 03 10. 22 33. 34 35. 76 7. 13513. 0 8911. 55311. 129 1. 767 1. 739 2. 781 2. 846 16.087 6.81 10.63 5.79 2.86813.978 9.030 2.451 1.793 1.226 2.464 1.422 66.517 4.51 3.41 3.73110.002 7.610 3.474 1.903 1.453 1.371 1.447 66.517 FHUIN FHC FHT FH+ FH- CL+ CL+ COMAX COMIN GOC GOT CI+ CI- WL-FT -10 5.47 12.42 21.74 22.14 1.67 1.70 10.67 11.07 5.01511.75619.93520.304 1.533 1.562 2.006 2.045 45.21 29.03 19.08 49.73 45.21 4.225 6.200 5.587 5.089 3.149 2.032 3.216 2.924 21.77 37.01 35.26 34.72 10.65 12.66 35.26 34.72 3.696 6.002 5.909 1.847 2.154 2.100 2.764 10.03 15.65 23.51 23.51 1.81 3.62 23.51 23.51 3.725 5.813 0.732 8.732 .672 1.343 2.764 2.764 3.79 10.85 12.66 1.45 1.81 10.85 12.66 3.066 3.475 9.95211.610 1.327 1.659 2.005 2.339 30.76 93.90 92.23 41.39 84.39 14.47 41.59 41.59 ,978 2.905 2.932 1.322 2.507 .460 2.962 2.062 20.73. 71.17 56.06 30.74 50.64 16.28 25.32 30.74 1.195 4.103 3.232 1.772 2.919 .938 2.346 2.848 163 1.00 4.00 13.37 36.24 27.13 17.36 23.51 9.04 16.28 17.36 1.806 4.695 3.664 2.345 3.175 1.221 2.300 2.462 2.10 6.00 16.71 107.09 115.74 28.94 34.04 18.08 36.17 28.94 4458 2.936 3.173 .793 2.578 .496 3.466 2.773 .42 6.00 A.36 33.61 28.94 10.45 21.70 10.85 9.04 7.96 1.505 6.050 5.209 1.953 3.907 1.953 2.221 1.954 167 1.34 6.00 11.76 70.84 66.91 10.00 57.87 14.47 20.94 19.00 .796 4.756 4.432 1.214 3.885 .971 4.340 2.712 .46 6.00 3.34 8.24 6.87 5.06 3.62 3.62 5.06 1.902 4.686 3.910 2.881 2.058 2.058 1.579 2.211 .62 4.00 5.35 12.36 11.39 9.75 6.33 3.80 9.04 9.95 1.927 4.452 4.105 3.584 2.281 1.368 2.094 2.304 Table 18 - Wave Force Tests Data and Calculated Results for Series 14; h = 4 ft. e = -45° 9. 3333FEET LENGTH OF PIPE= 32.01 18.39 6.81 14.39 15.01 2.95410.684 6.775 4.324 2.484 200 FH- CL+ CL- CD4AX COMIN TOTAL VOLUME: 1.8017 FT3 FHMAX FHMIN FHC FHT FH+ 2.55 21.07 3.34 FLANG: ANGLE =- 45.00EGREE 55.17 13.62 52.75 13.52 4.41 37.61 55.19 49.73 41.92 76.91 57.84 65.62 197.77 167.20 79.11 50.16 26.37 22.74 43.47 33.5- 146.50 107.01 45.17 157.44 130.41 17.54 13.34 ... F ... 34.79 16.04 77.64 734.40 - 1. . SERTES NO.14. 2UN 157-169 WEST DINAMONITER .... EAST DIMMON-TER 3.34 21.47 10.21 27.70 6.36 21.47 MATER DEPTH= 4.0FE: T 24.4 1.54 4.00 161 2.07 4.00 1.64 2.00 2.00 3.59 2.47 4.16 3.69 1.24 5.44 1.16 1.33 2.00 1.55 1.91 2.00 . 66 1.11 1.41 59.2 1.02 1.97 162 151 159 165 300 151 159 191 162 163 165 167 161

FIG. 1 - Split Pipe



Assembled Sections



Split Pipe Cable Protector



FIG. 2 - The OSU Wave Research Facility

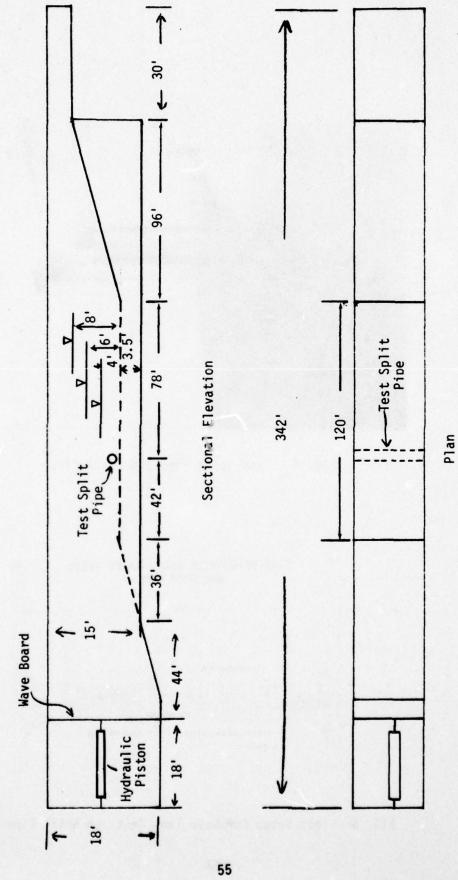


FIG. 3 - Location of Test Split Pipe and False Floor Configuration

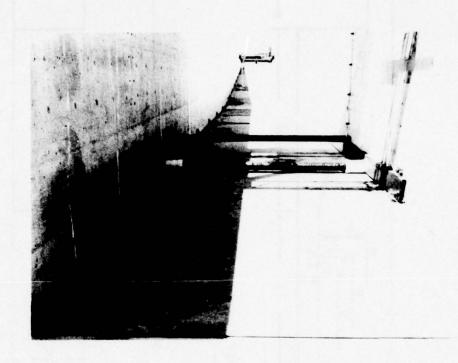


FIG. 4 - Test Split Pipe and False Floor

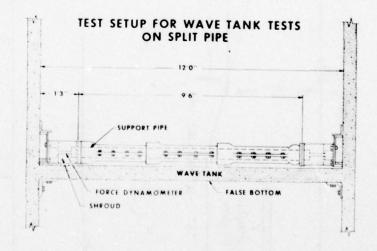


FIG. 5 - Test Setup for Wave Tank Tests on Split Pipe

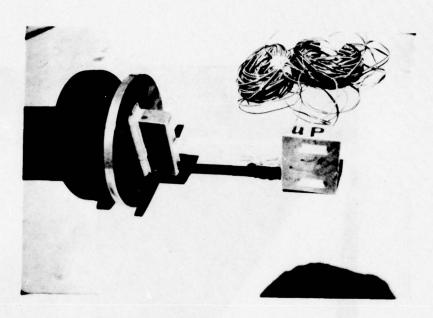


FIG. 6 - Force Dynamometer

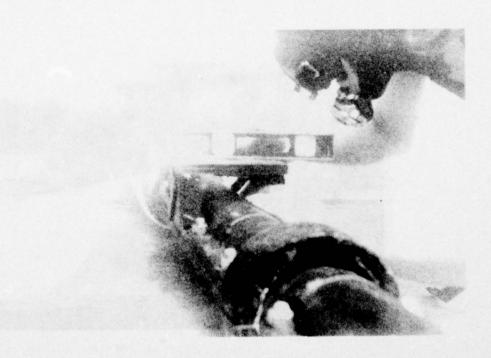


FIG. 7 - Underwater Operation of Changing of Flange Angle

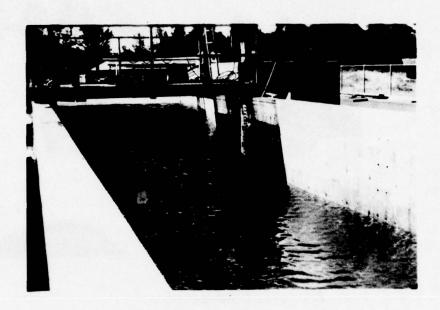


FIG. 8 - The Test Wave With T = 6 sec., H = 3.3 ft., h = 8 ft.



FIG. 9 - The Test Wave With T = 2 sec., H = 1.9 ft., h = 8 ft.

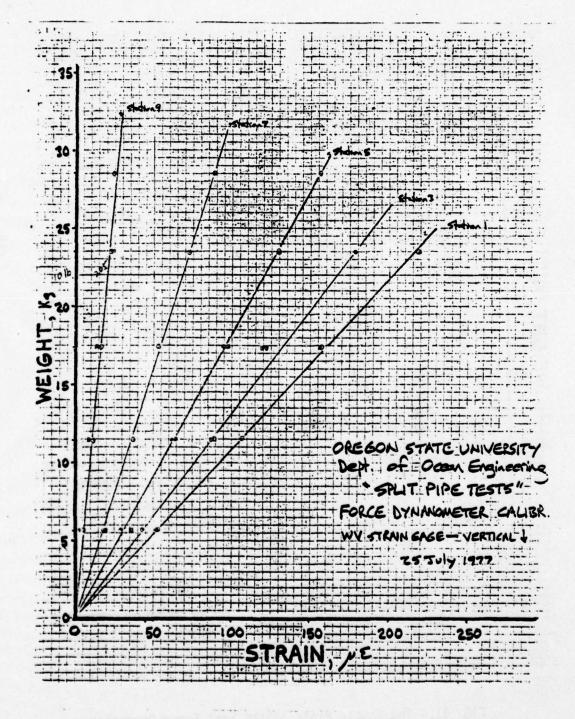


FIG. 10 - The Sample Plots of the West Force Dynamometer Output vs. Weight

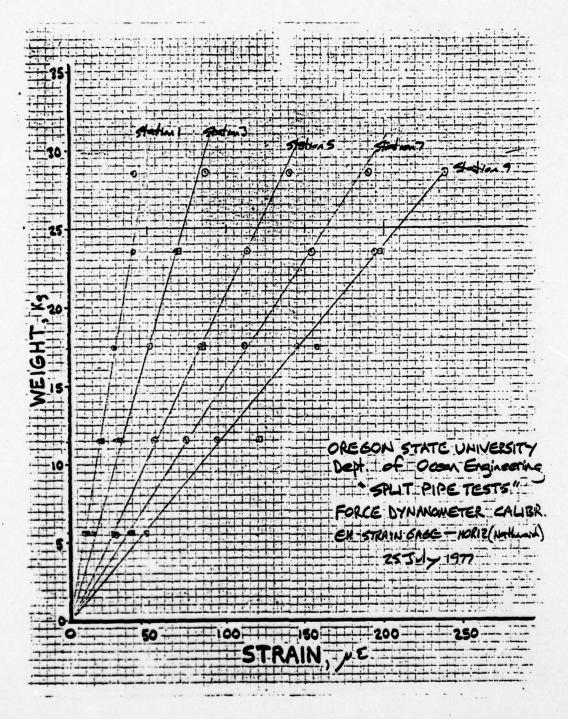


FIG. 11 - The Sample Plots of the East Force Dynamometer Output vs. Weight

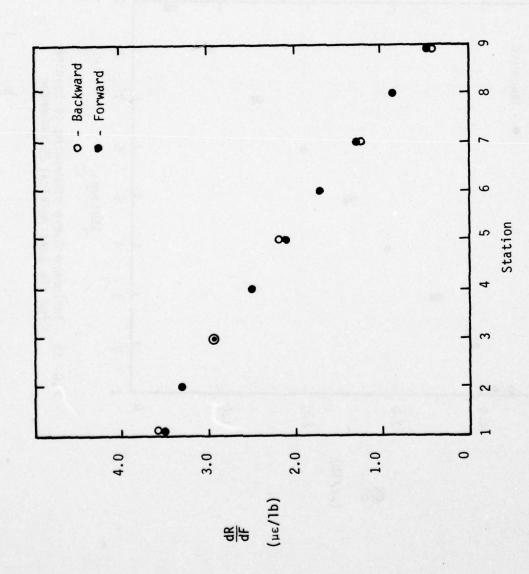


FIG. 12 - Influence Curve of Reading to Loading for the West Horizontal Dynamometer

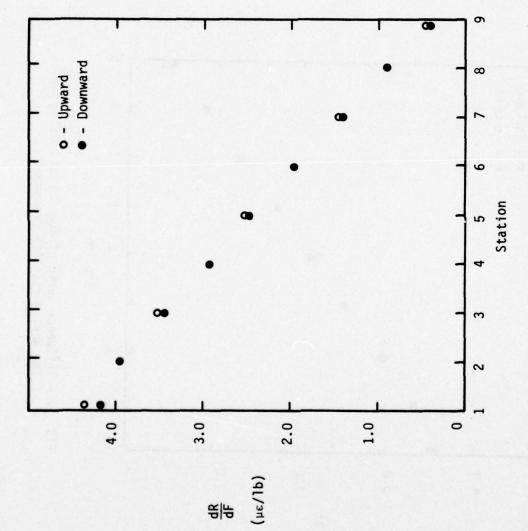
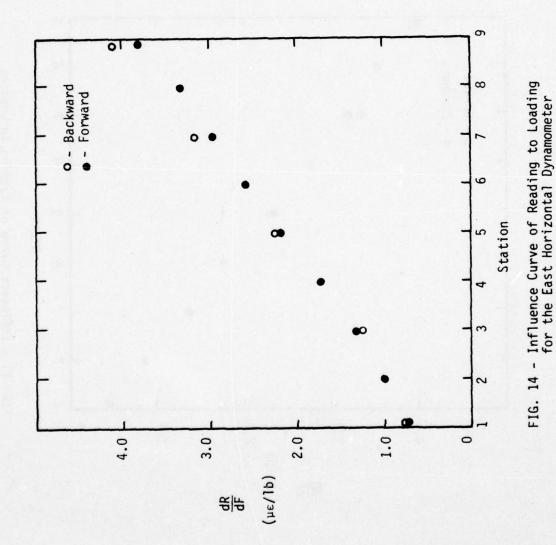


FIG. 13 - Influence Curve of Reading to Loading for the West Vertical Dynamometer



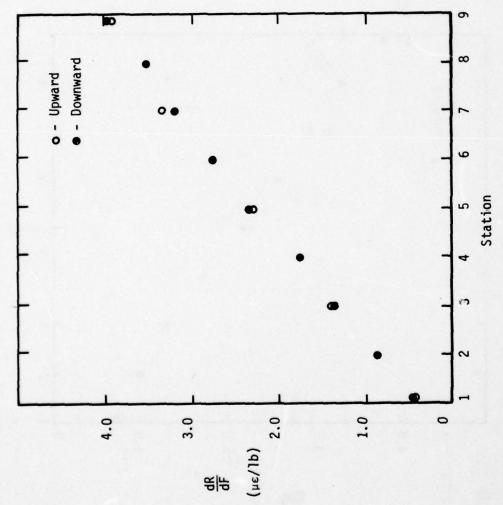


FIG. 15 - Influence Curve of Reading to Loading for the East Vertical Dynamometer

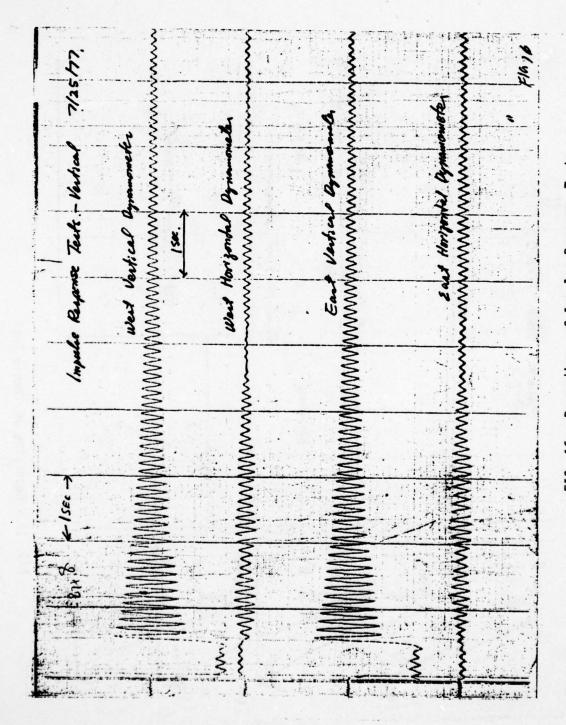


FIG. 16 - Recording of Impulse Response Tests

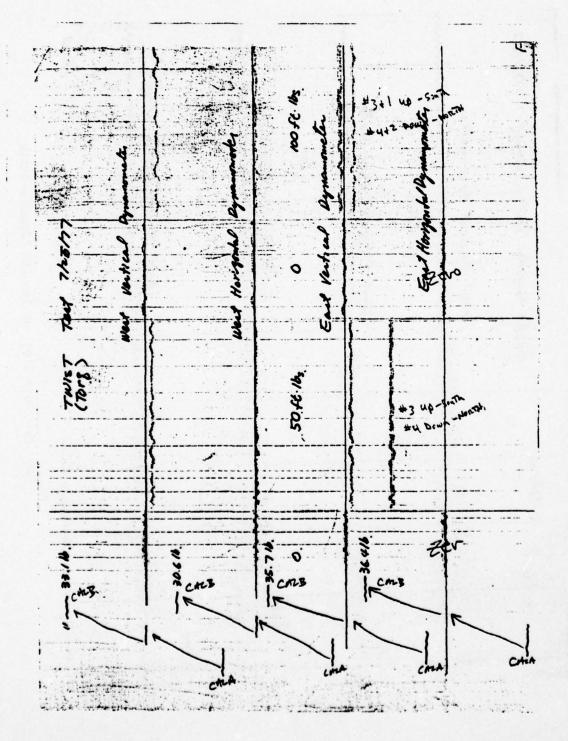


FIG. 17 - Recording of Torque Test

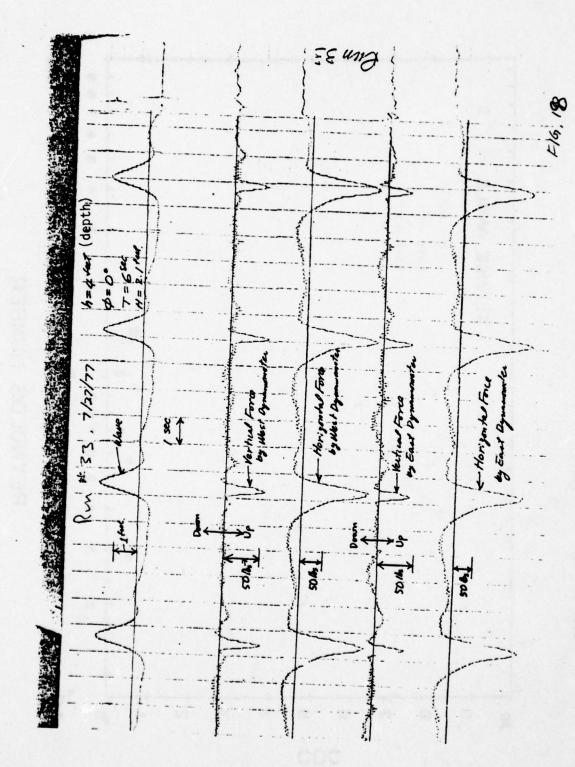


FIG. 18 - Example Recording of Wave Force Test, Run 33

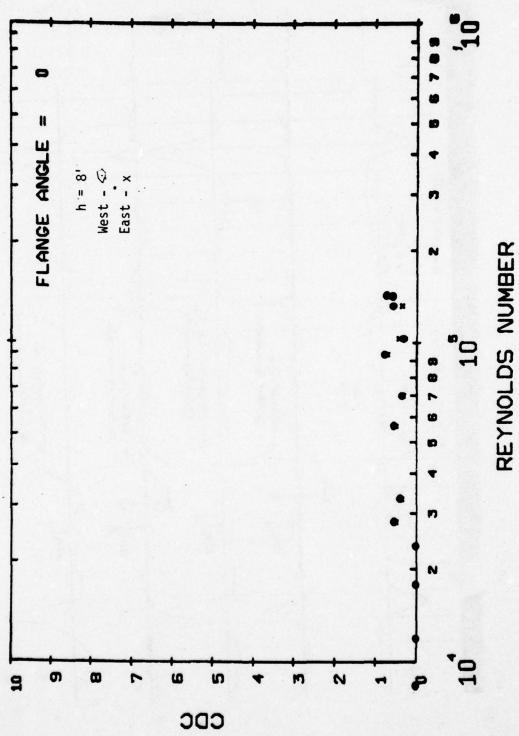


FIG. 19 - Comparison Between West Dynamometer and East Dynamometer for CDC vs. Re,  $\phi$  = 00, h = 8'

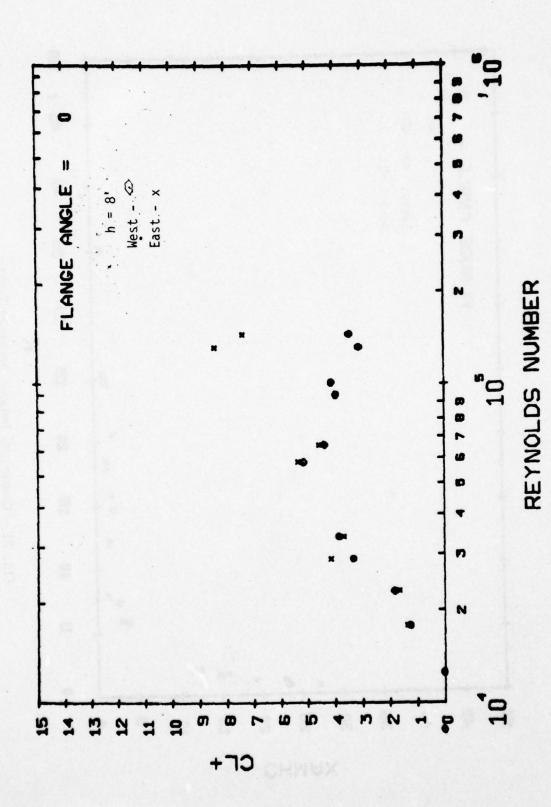
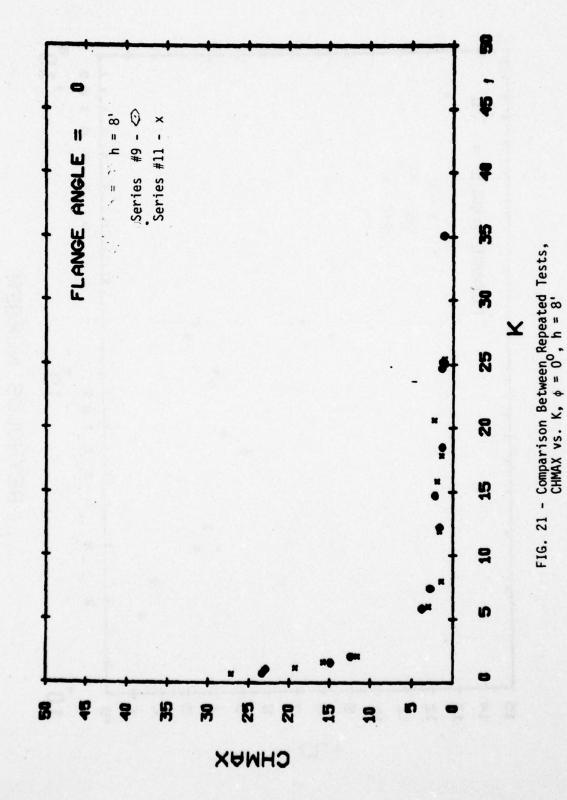


FIG. 20 - Comparison Between West Dynamometer and East Dynamometer for CDC vs. CL+ vs. Re,  $\phi$  = 45°, h = 8'

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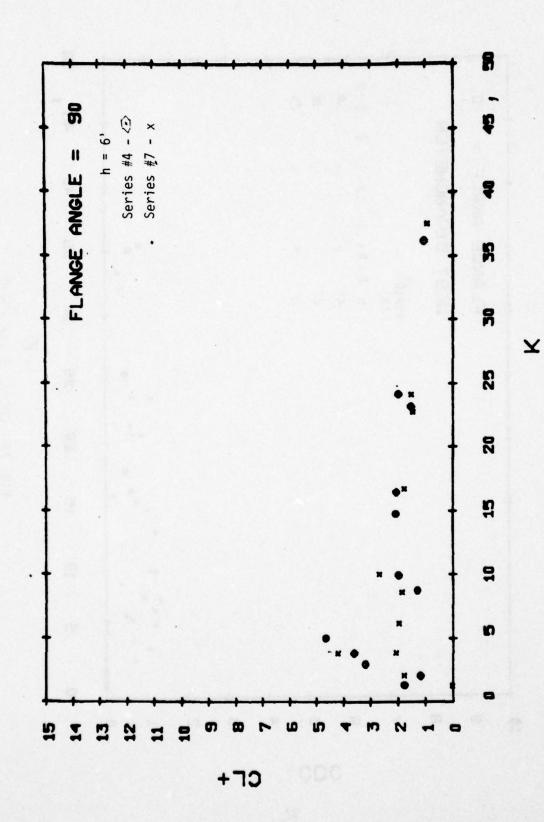
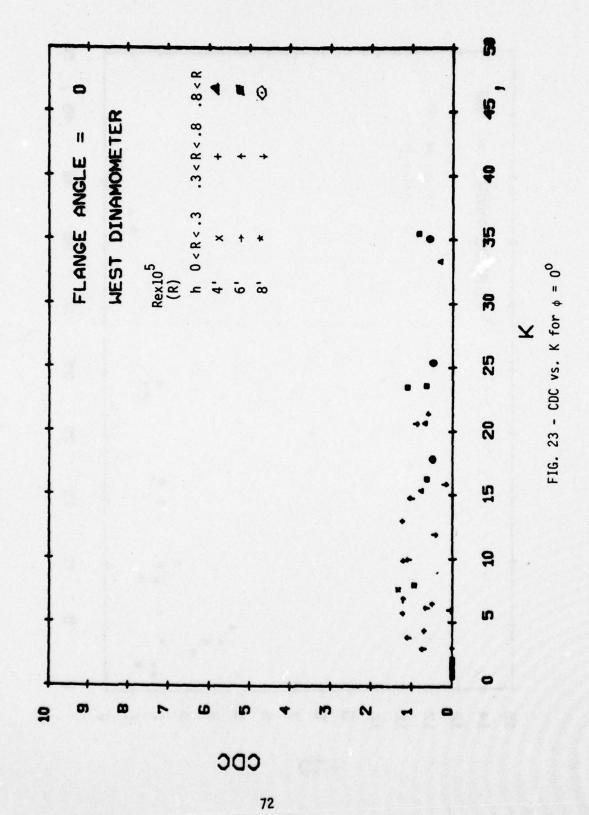


FIG. 22 - Comparison Between Repeated Tests, CL+ vs. K,  $\phi$  = 90°, h = 6'



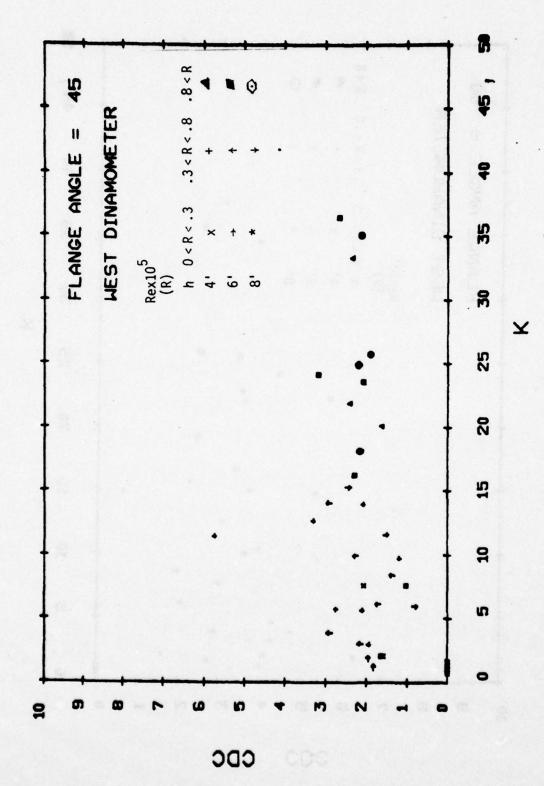
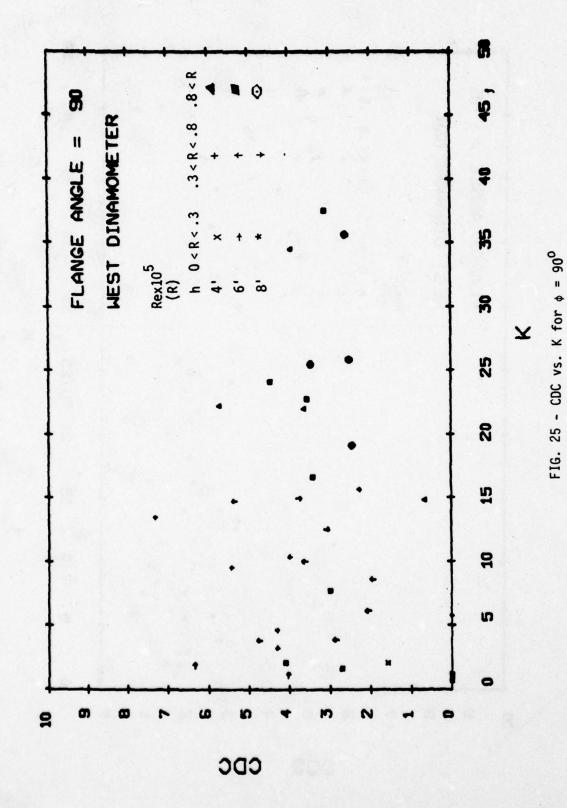
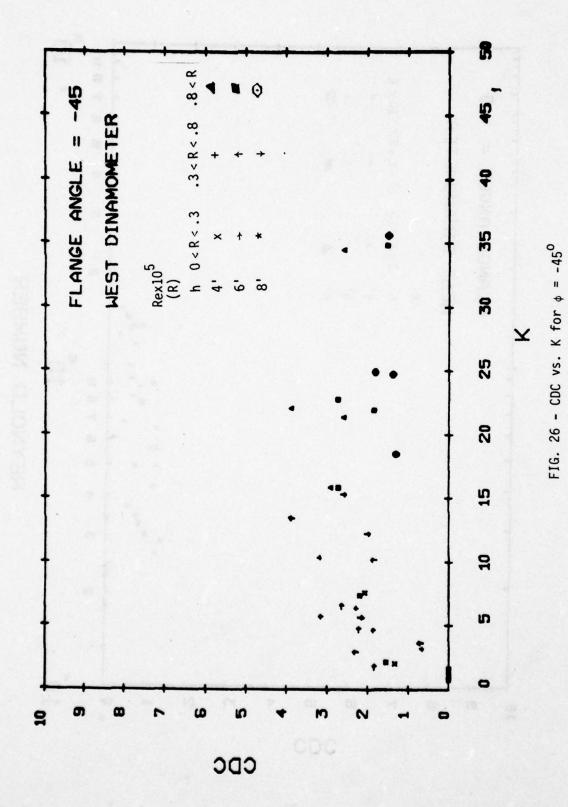


FIG. 24 - CDC vs. K for  $\phi$  = 45°

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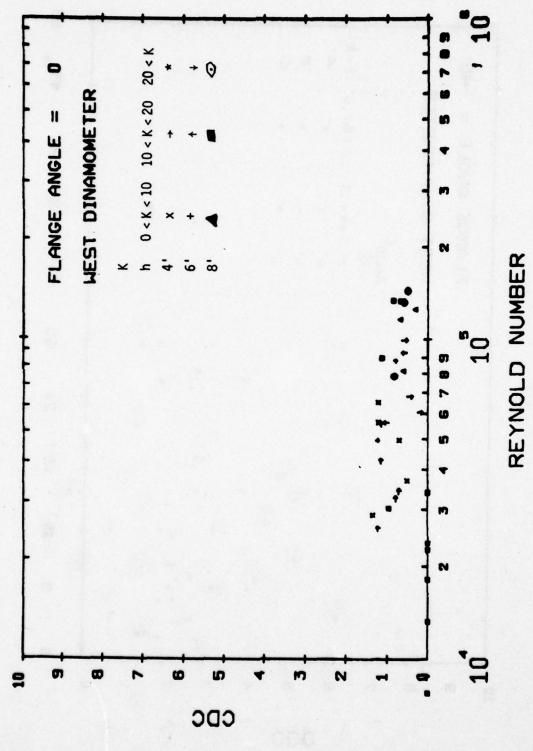
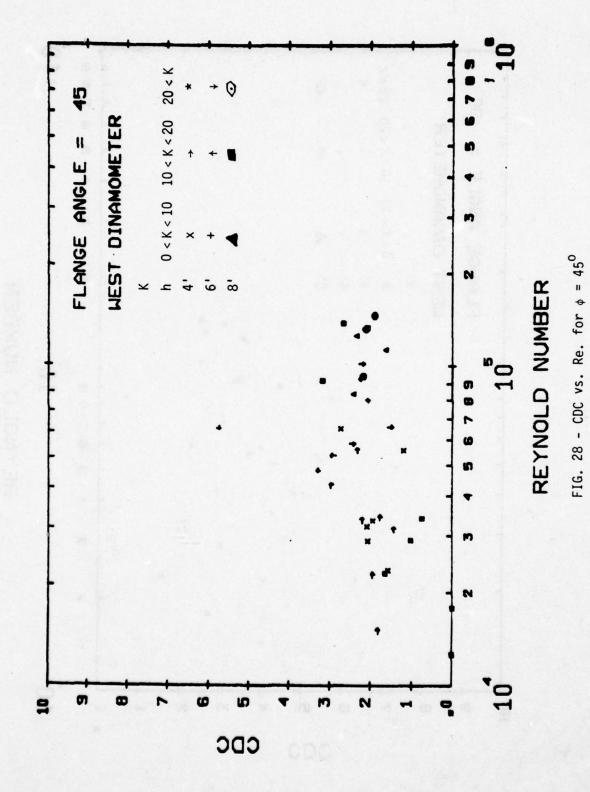
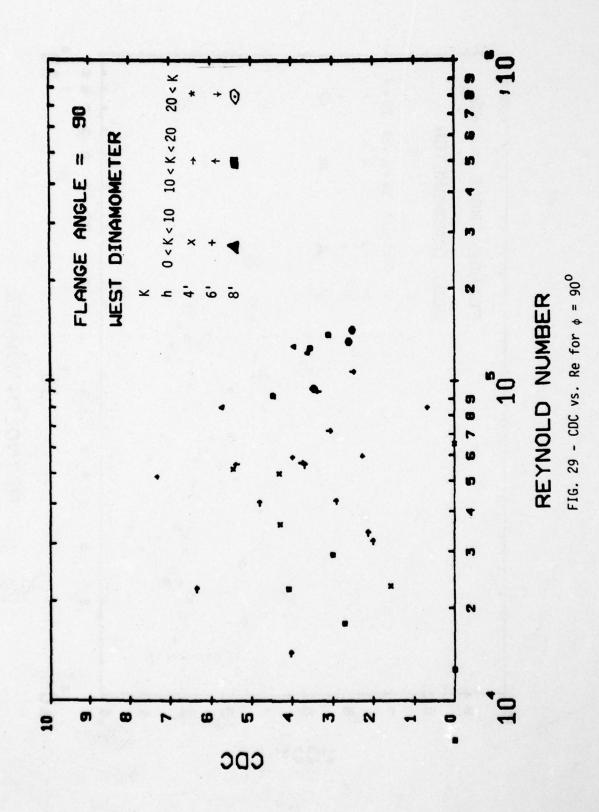
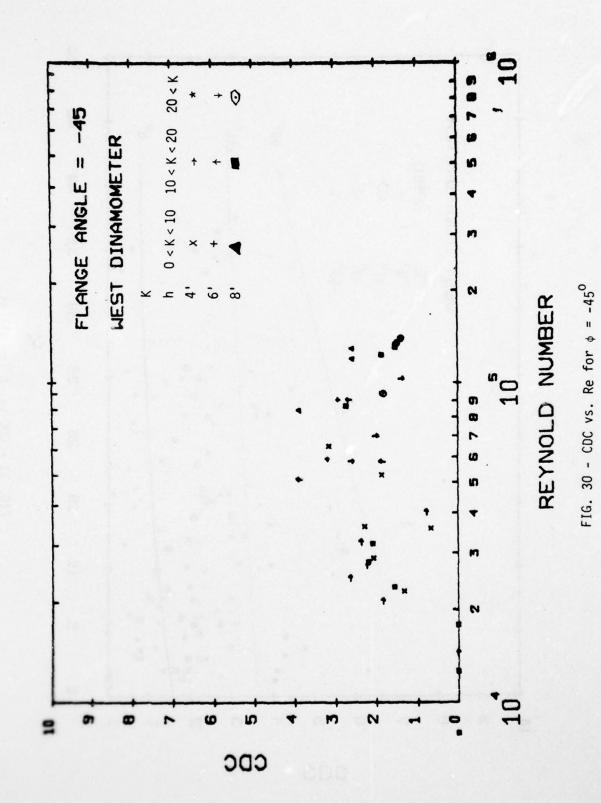


FIG. 27 - CDC vs. Re for  $\phi = 0^{\circ}$ 







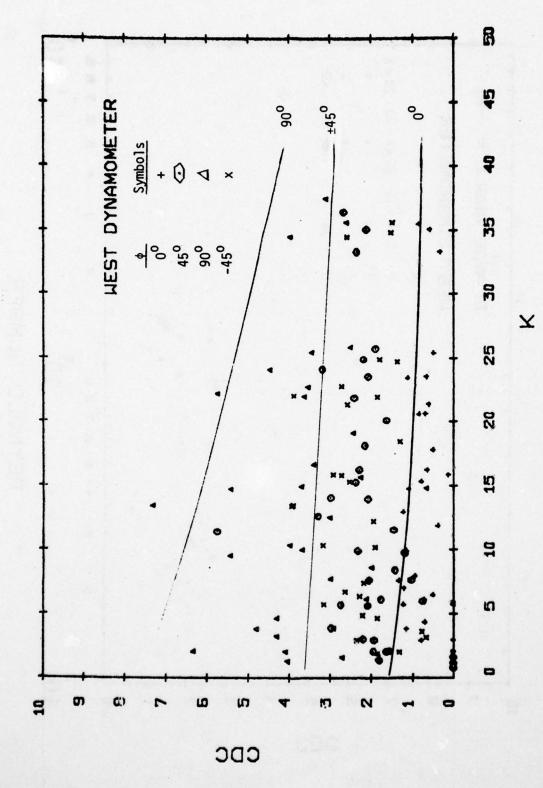


FIG. 31 - CDC vs. K for all \$s

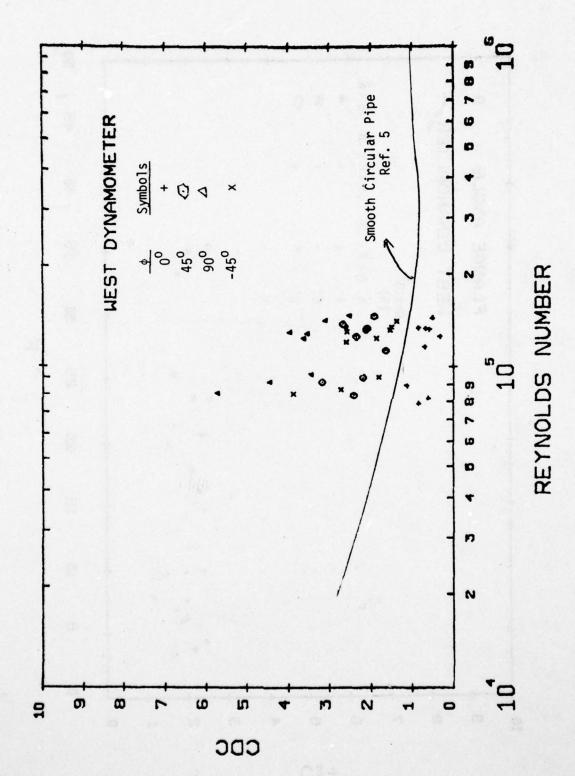
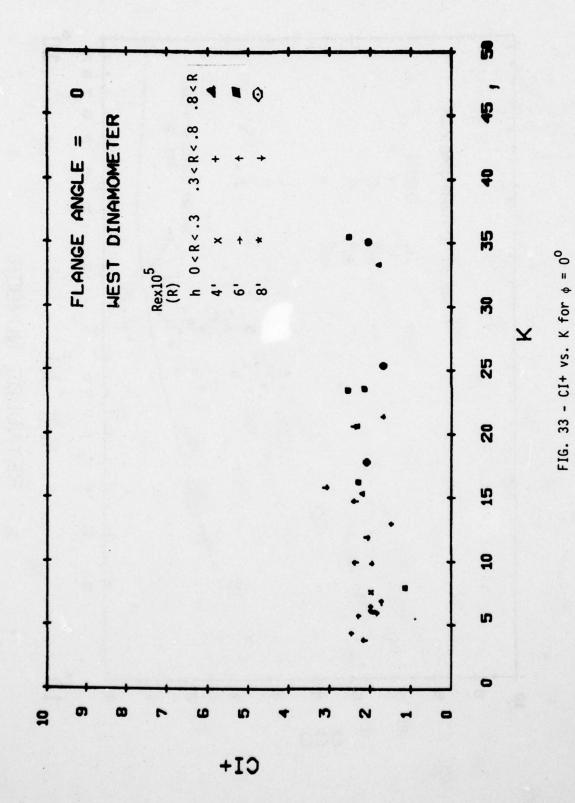
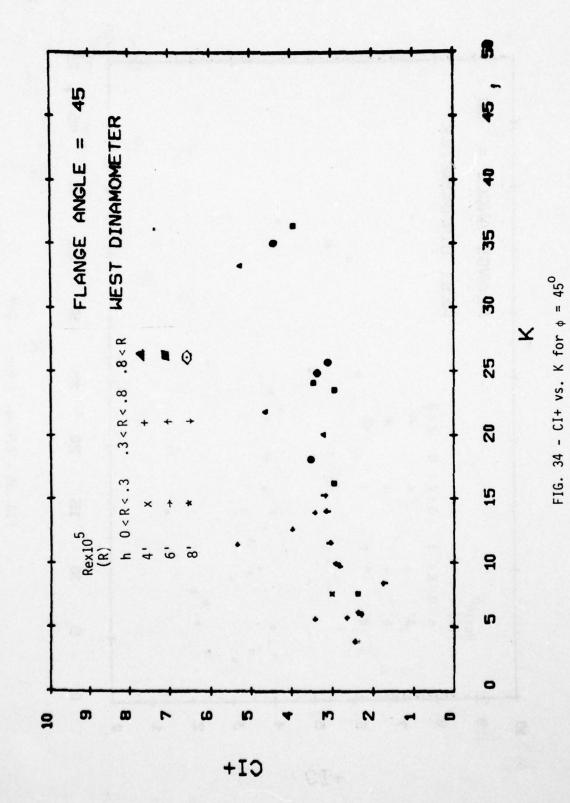
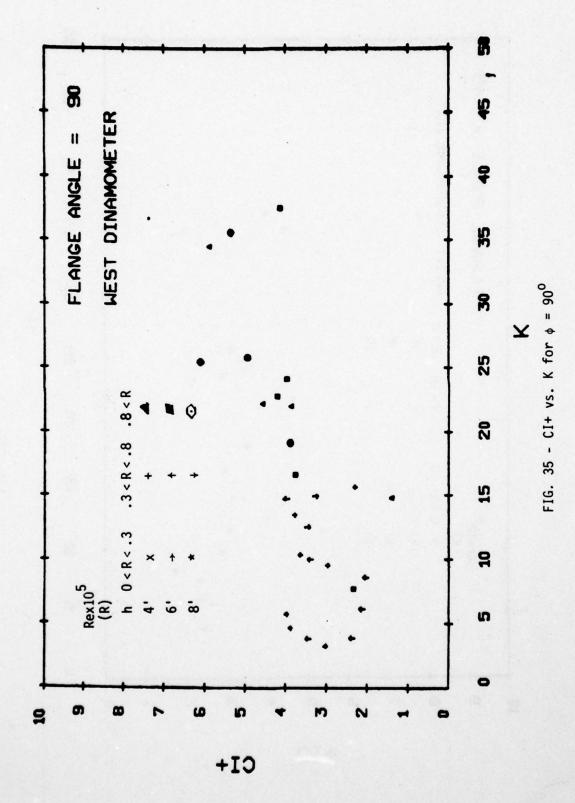


FIG. 32 - CDC vs. Re for all \$\$, K >20







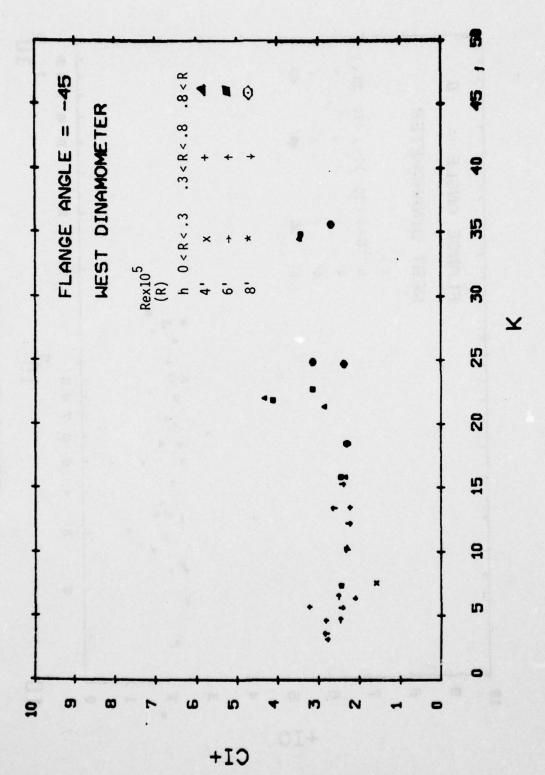


FIG. 36 - CI+ vs. K for  $\phi$  = -45°

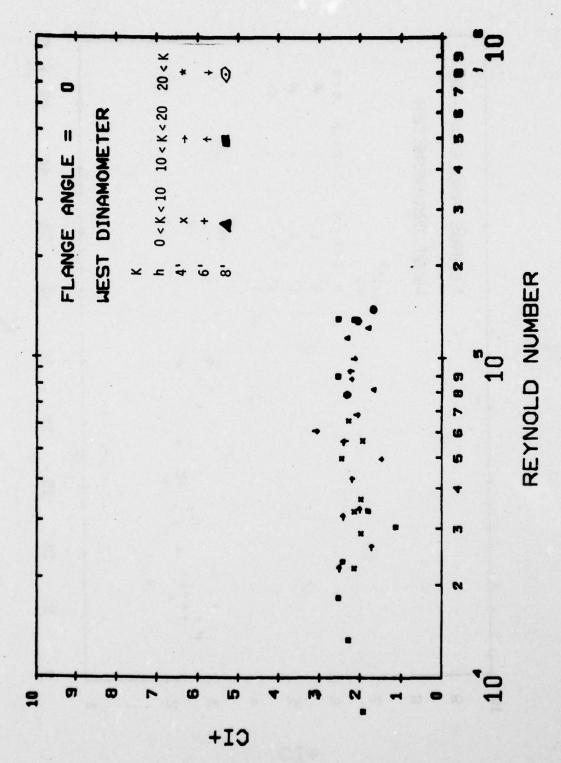
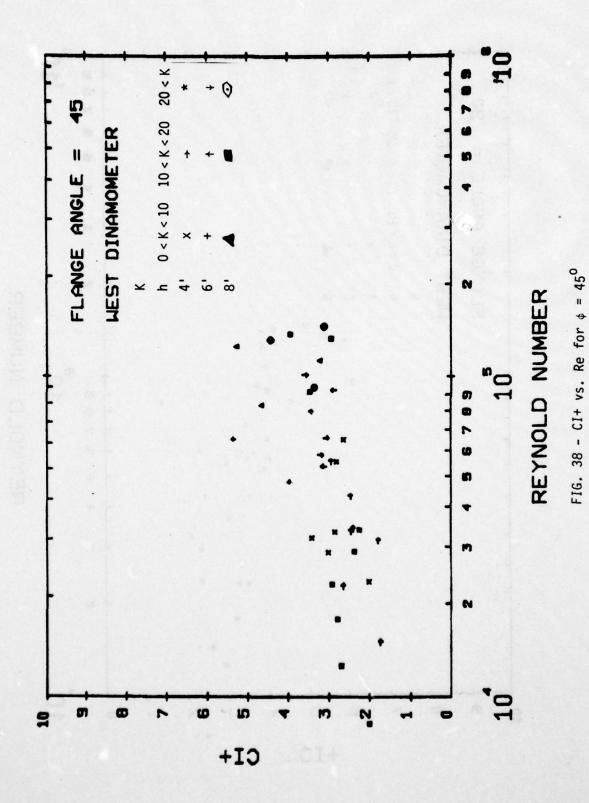
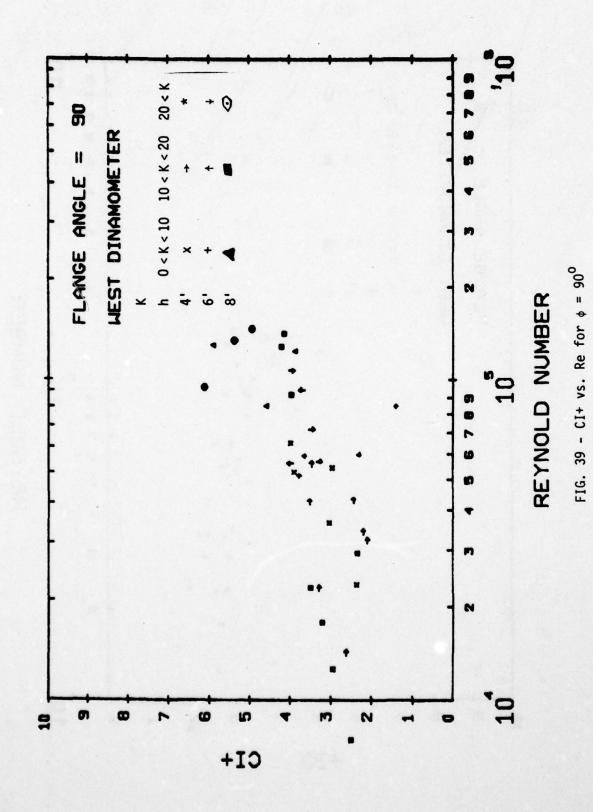
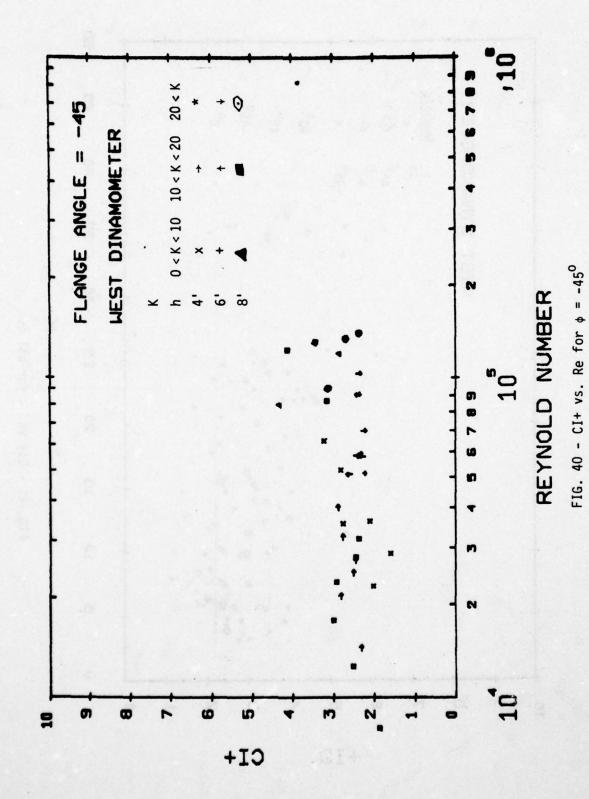
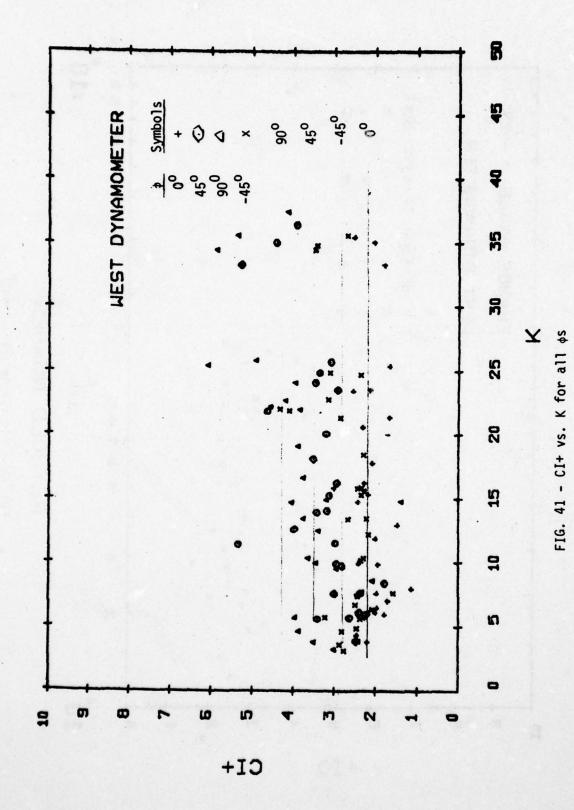


FIG. 37 - CI+ vs. Re for  $\phi = 0^{\circ}$ 









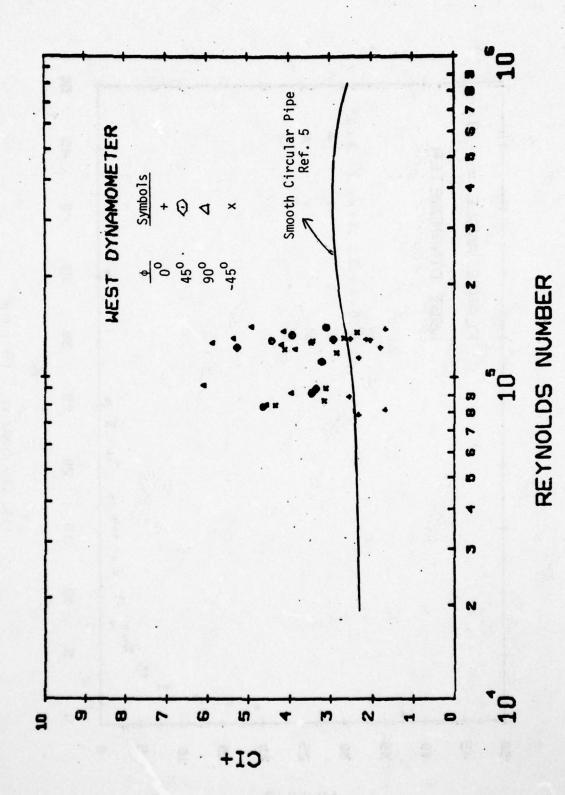
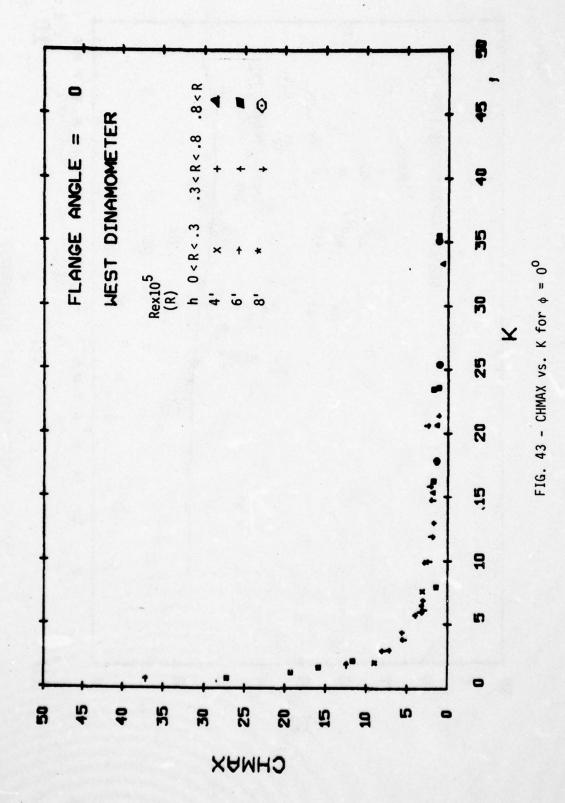
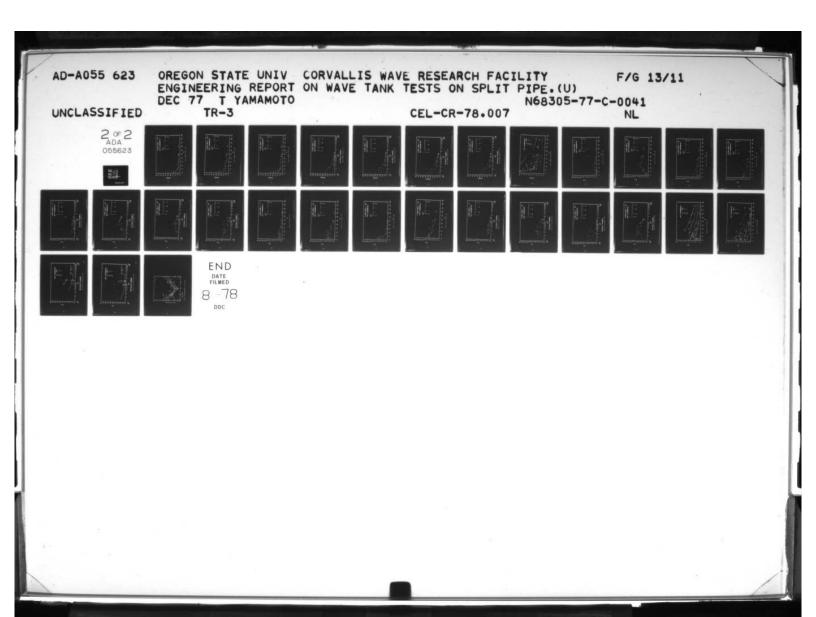
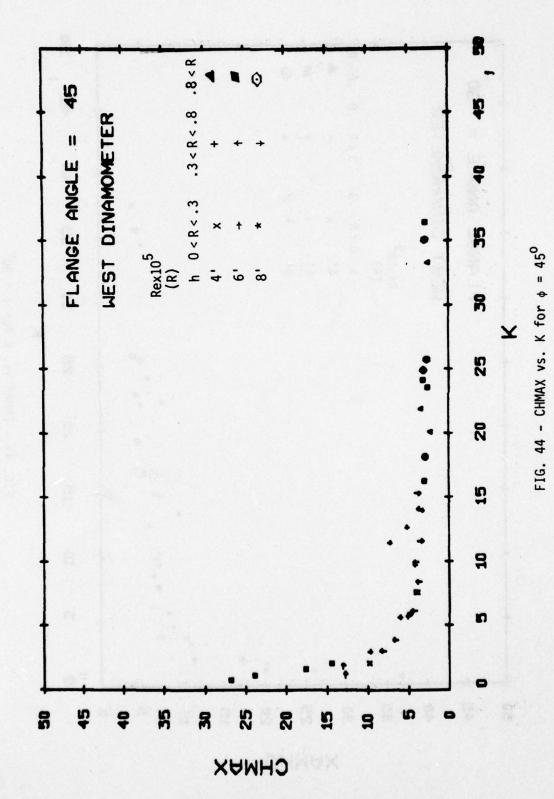


FIG. 42 - CI+ vs. Re for all \$\$, K >20









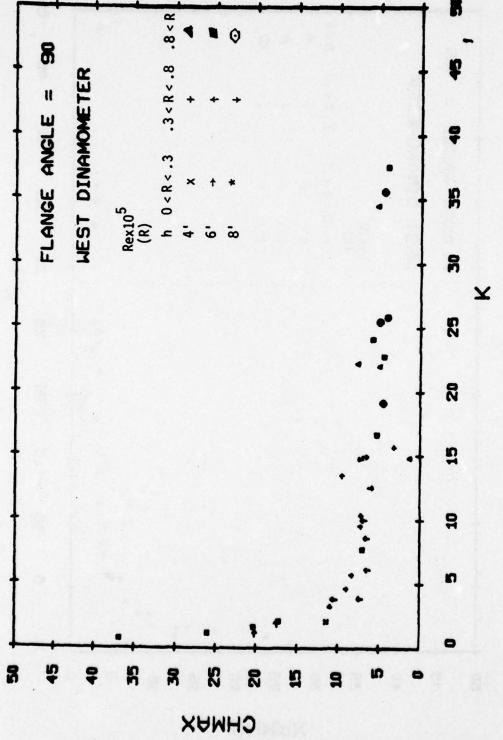


FIG. 45 - CHMAX vs. K for  $\phi = 90^{\circ}$ 



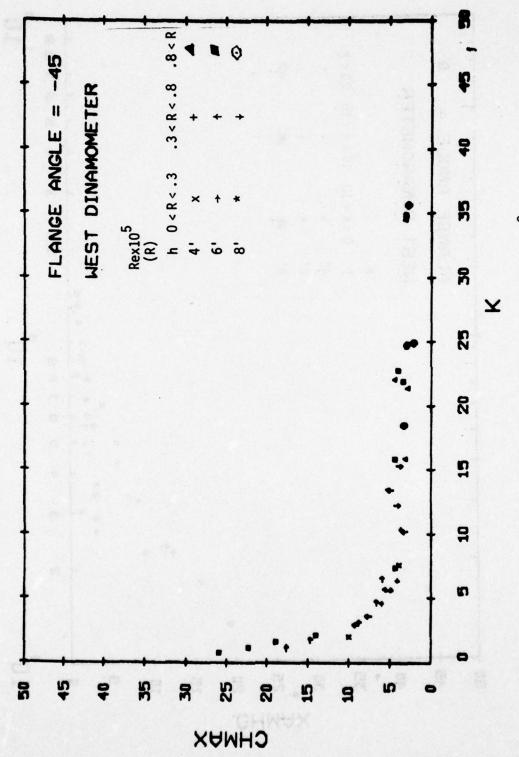


FIG. 46 - CHMAX vs. K for  $\phi$  = -45°

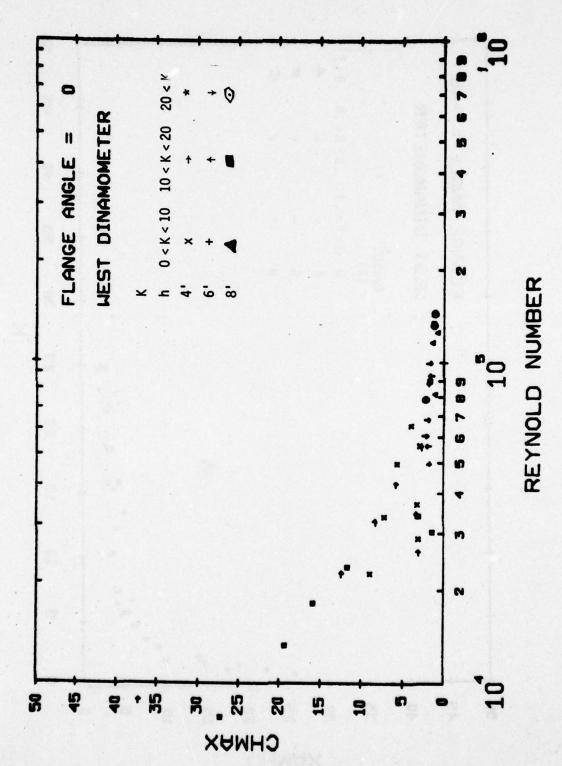
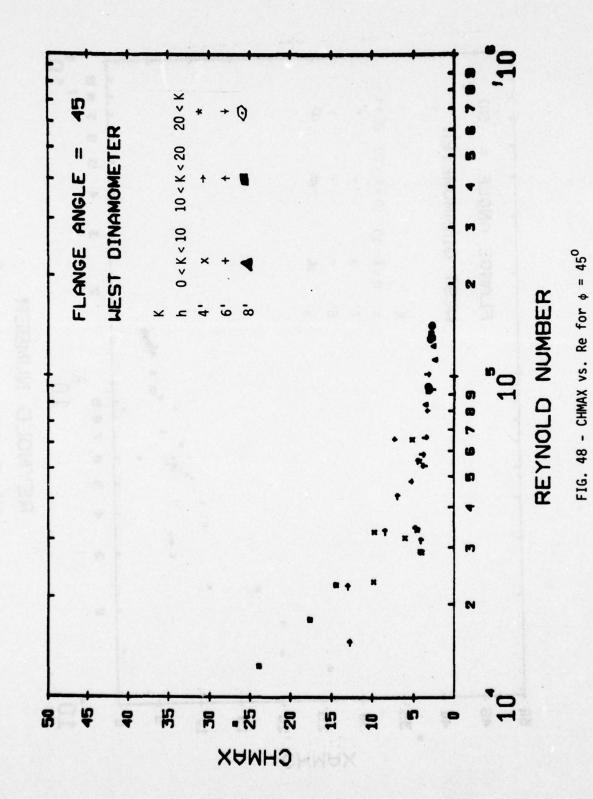
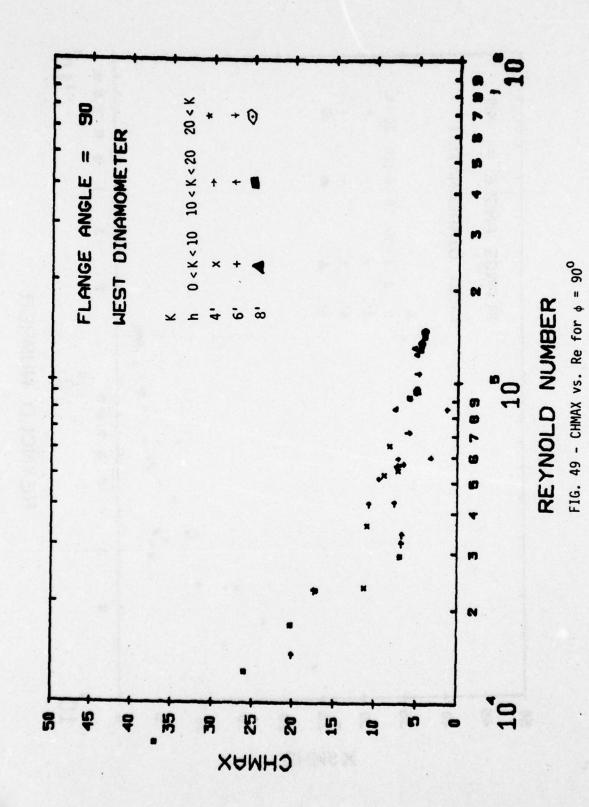


FIG. 47 - CHMAX vs. Re for  $\phi = 0^{\circ}$ 



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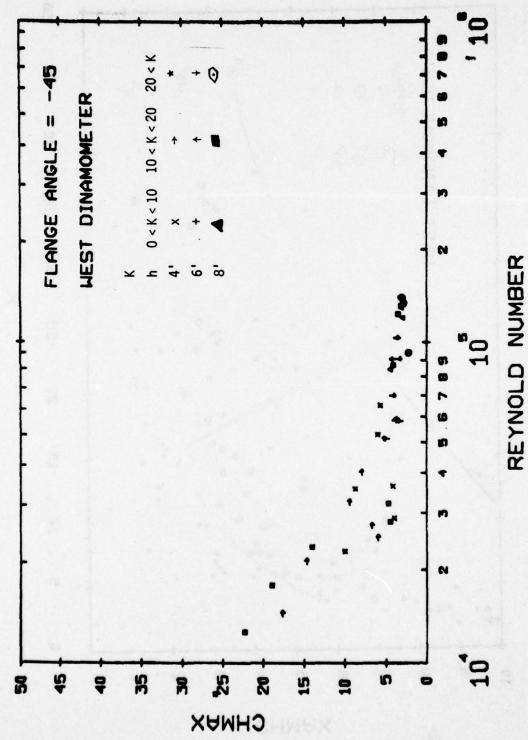


FIG. 50 - CHMAX vs. Re for  $\phi = -45^{\circ}$ 

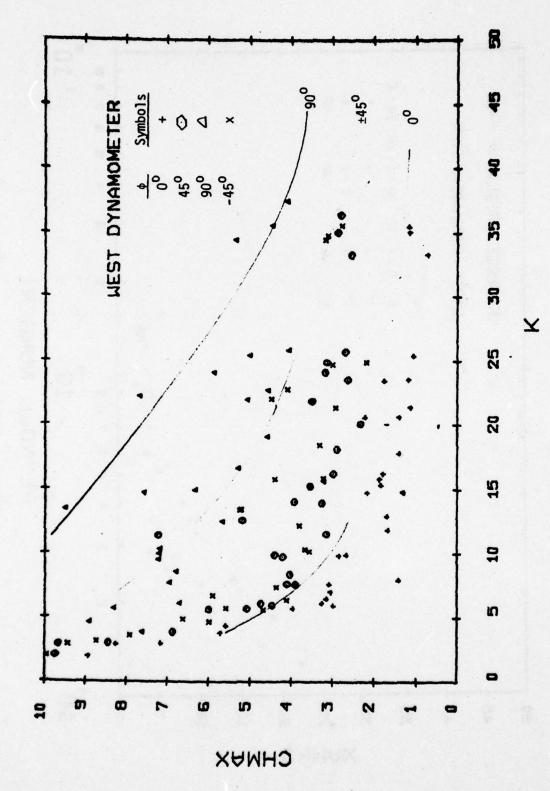
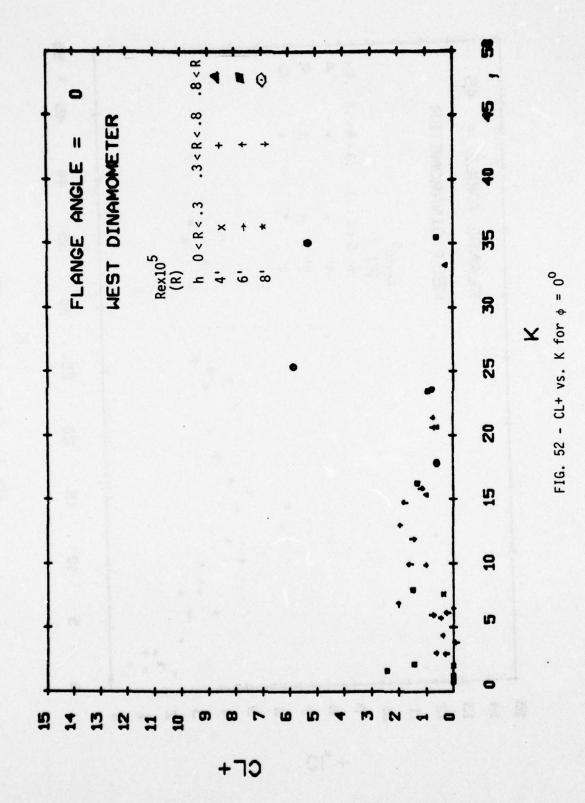
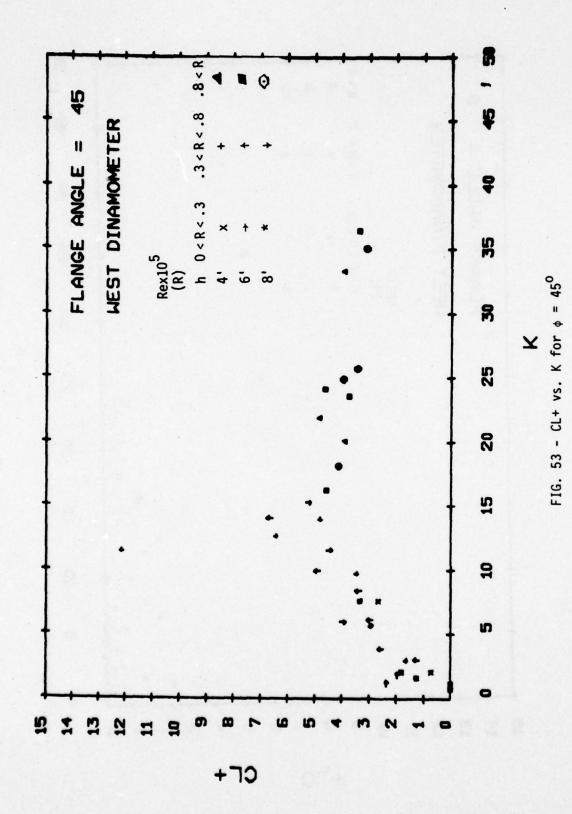
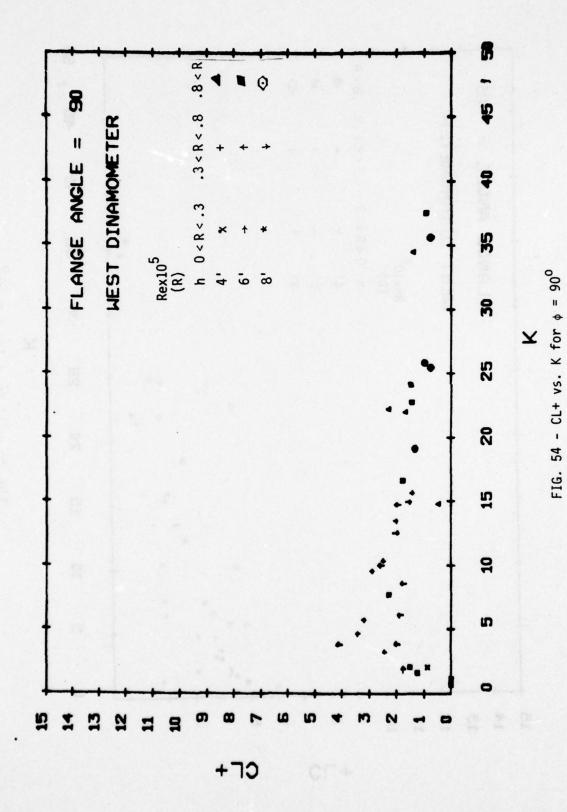
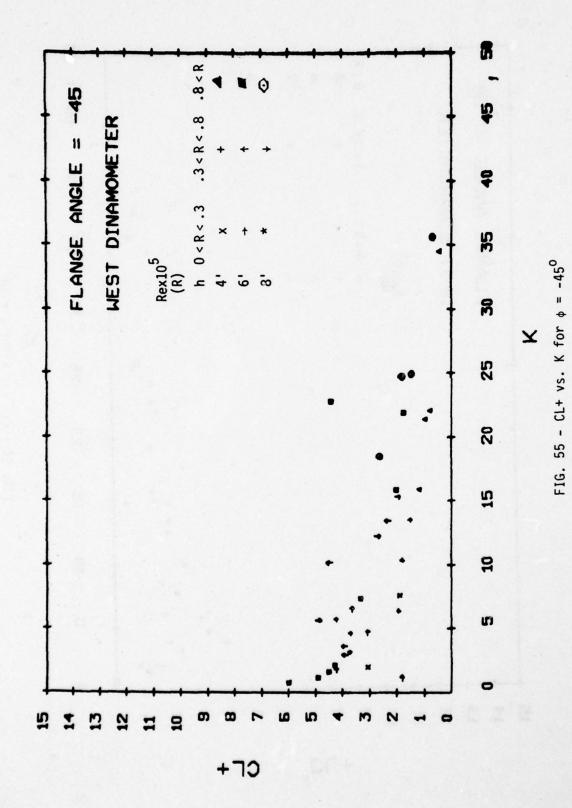


FIG. 51 - CHMAX vs. Re for all ¢s



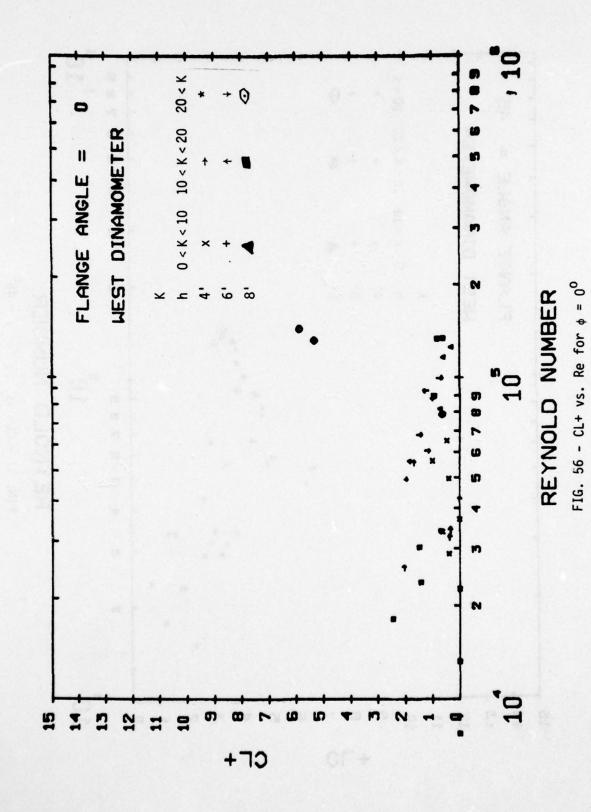


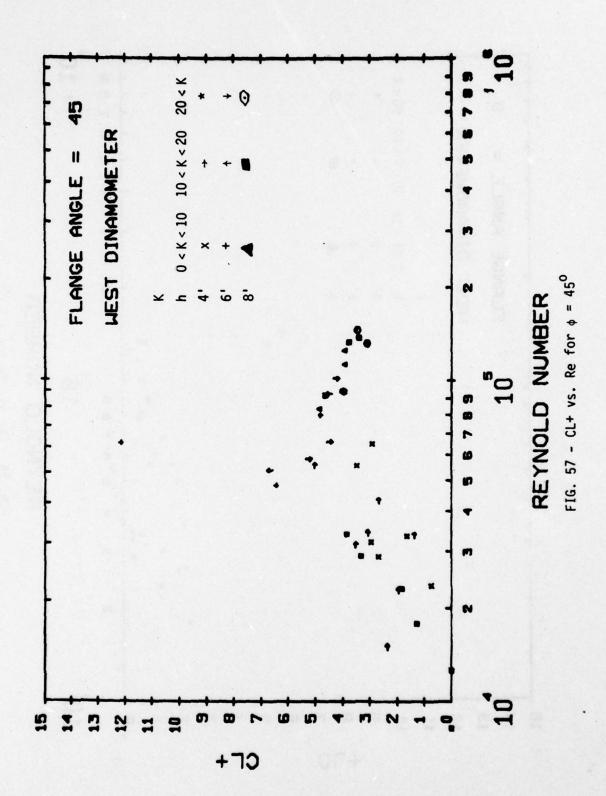


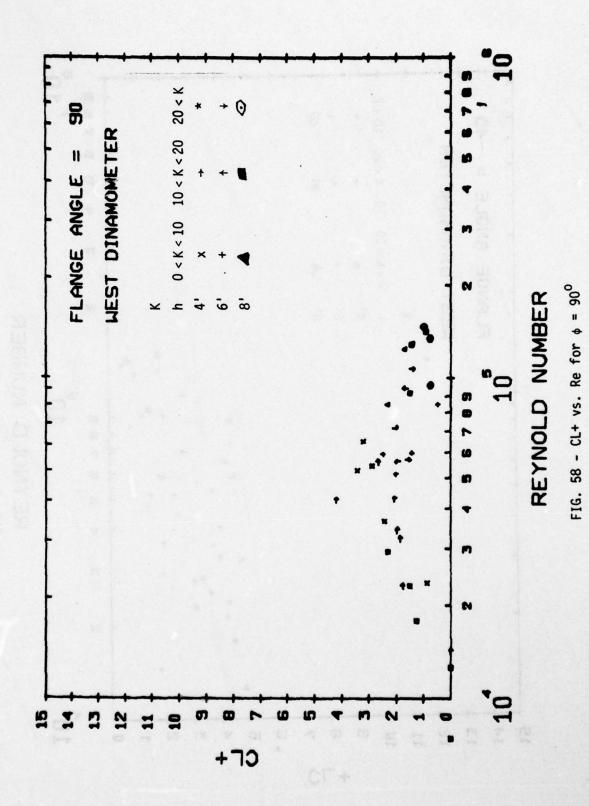


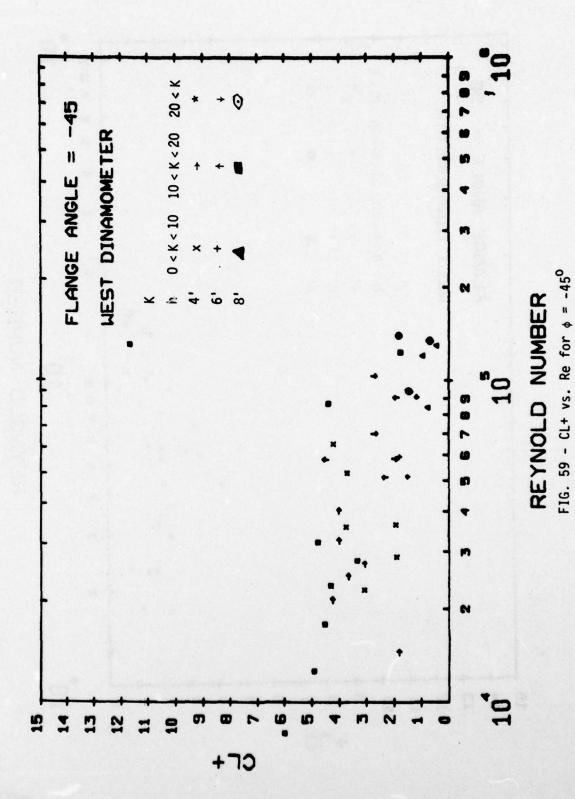
Sandri . Cadel Plane

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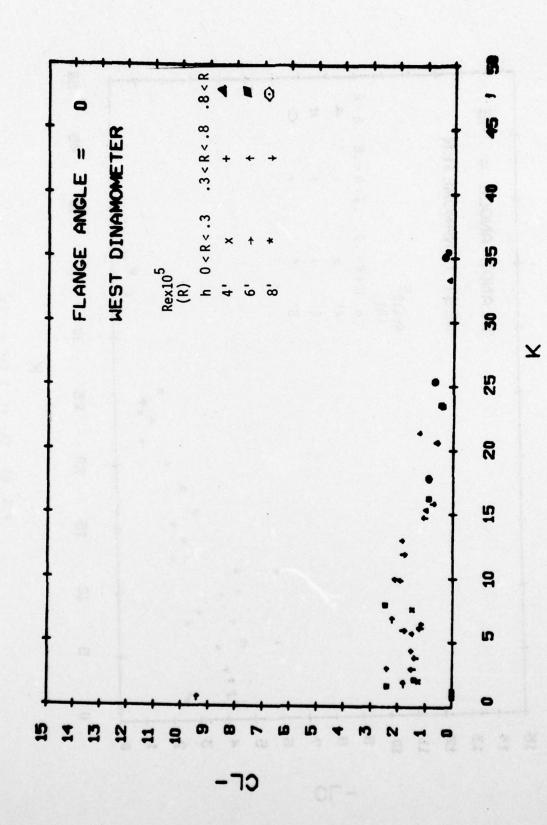
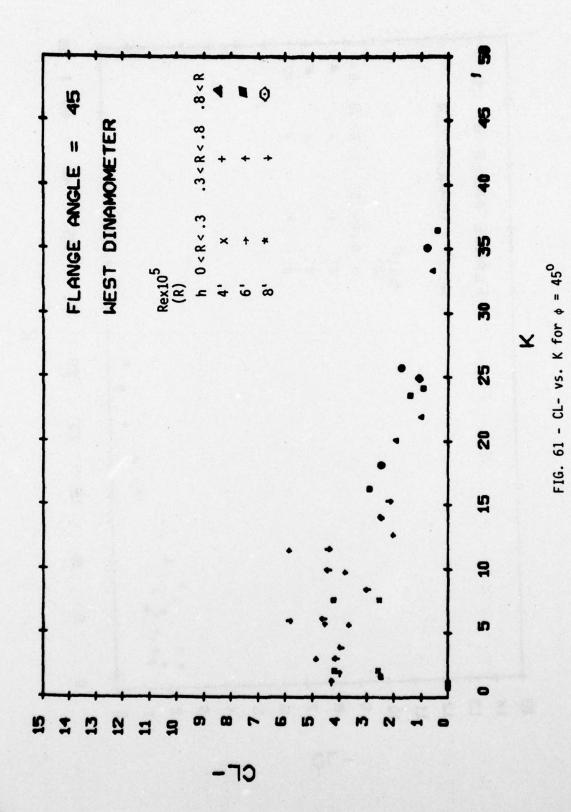
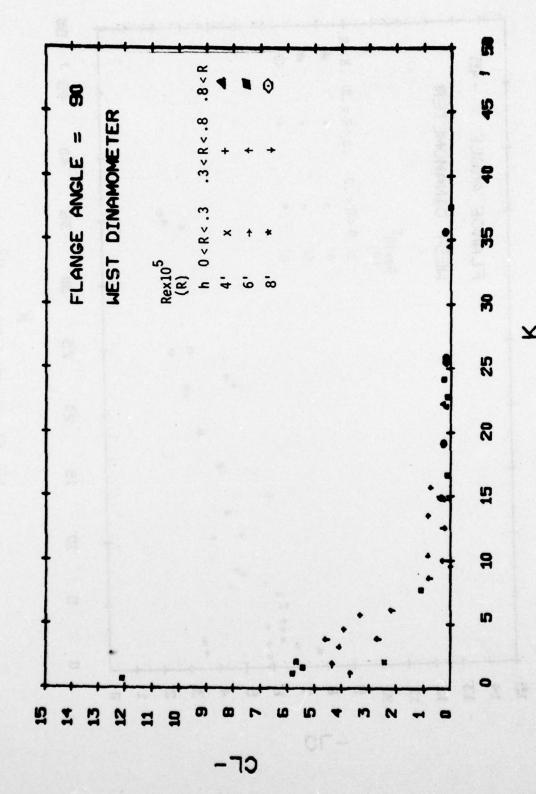
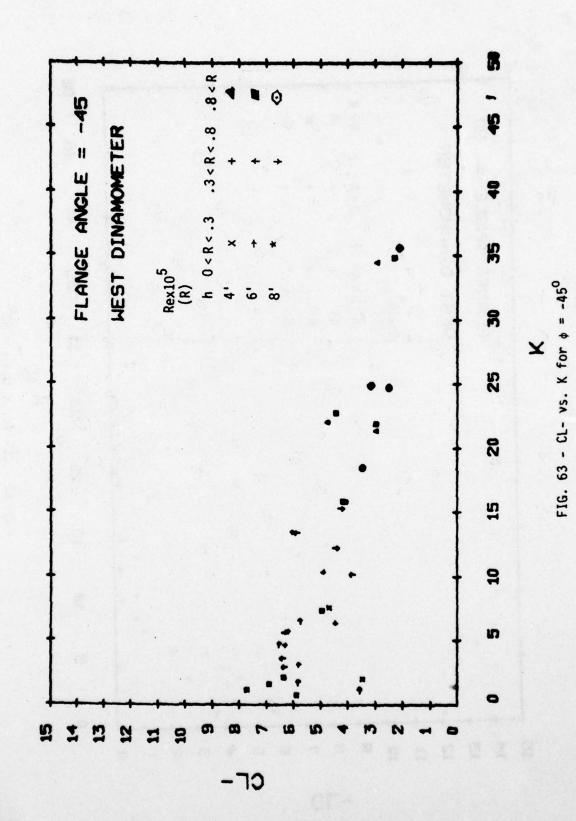


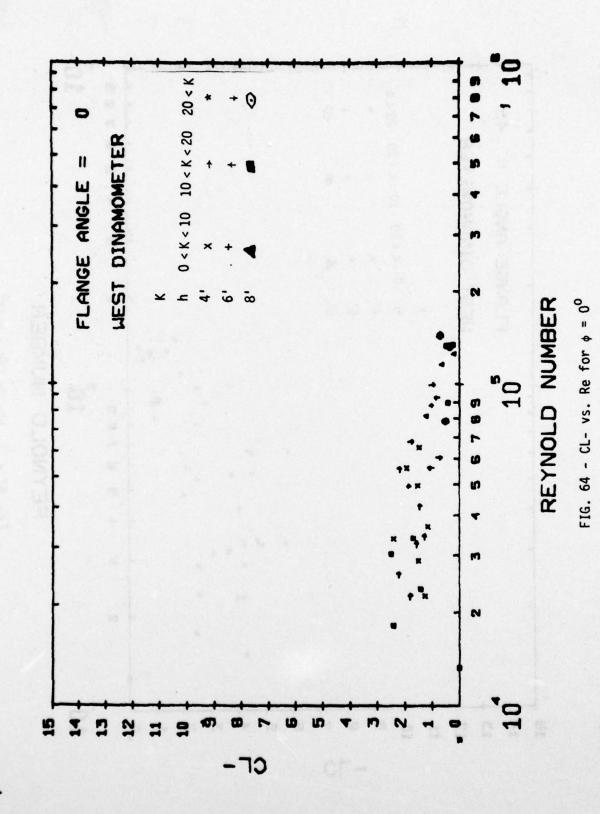
FIG. 60 - CL- vs. K for  $\phi = 0^{0}$ 

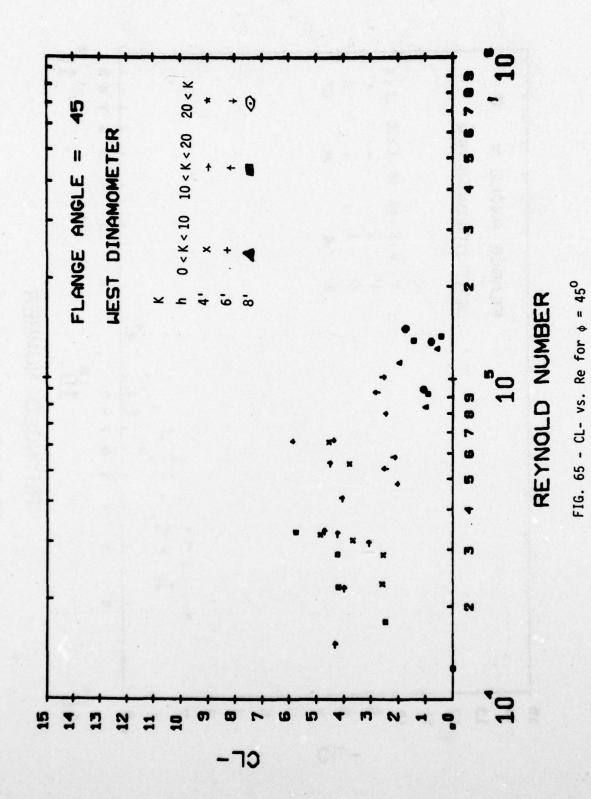


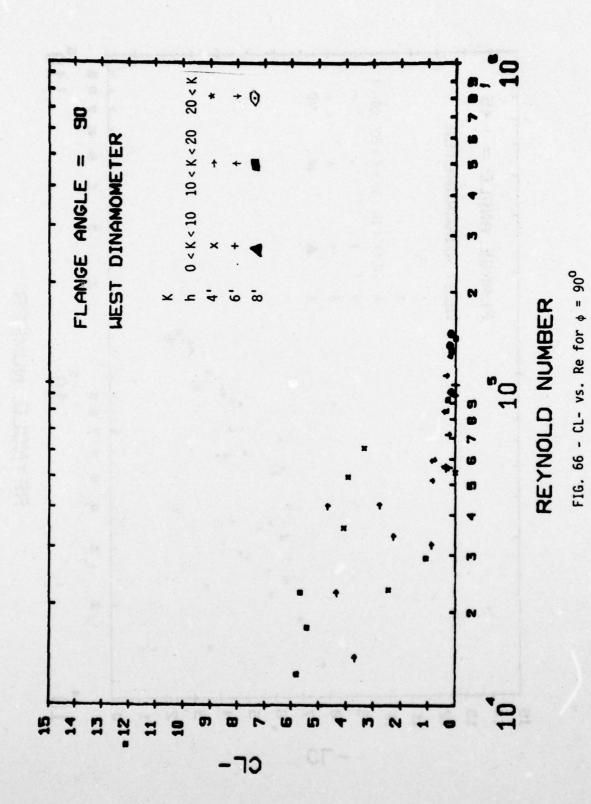


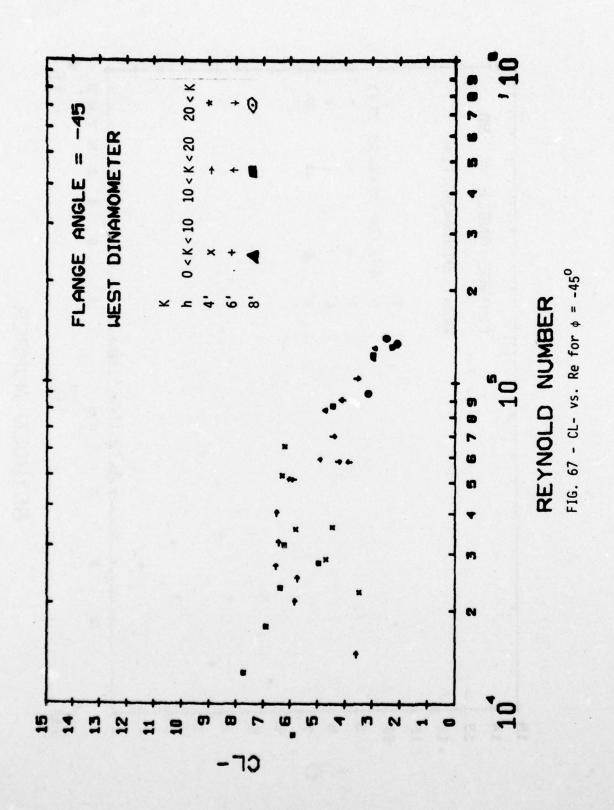
**K** FIG. 62 - CL- vs. K for  $\phi = 90^{\circ}$ 

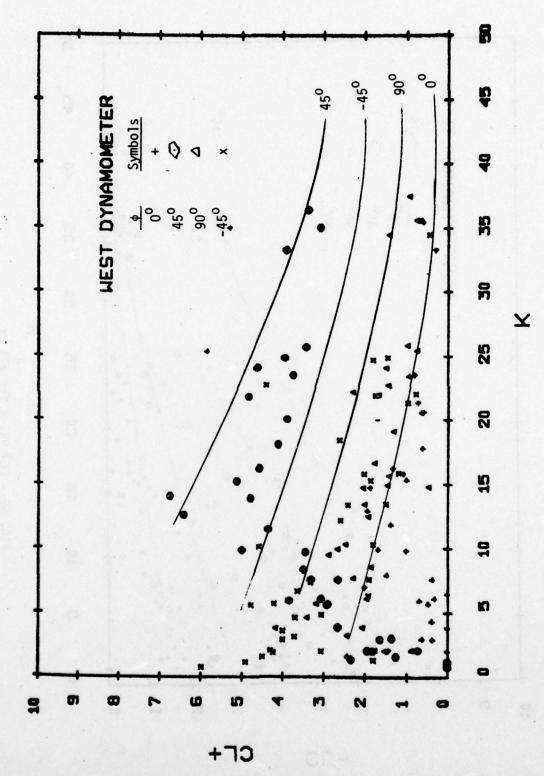


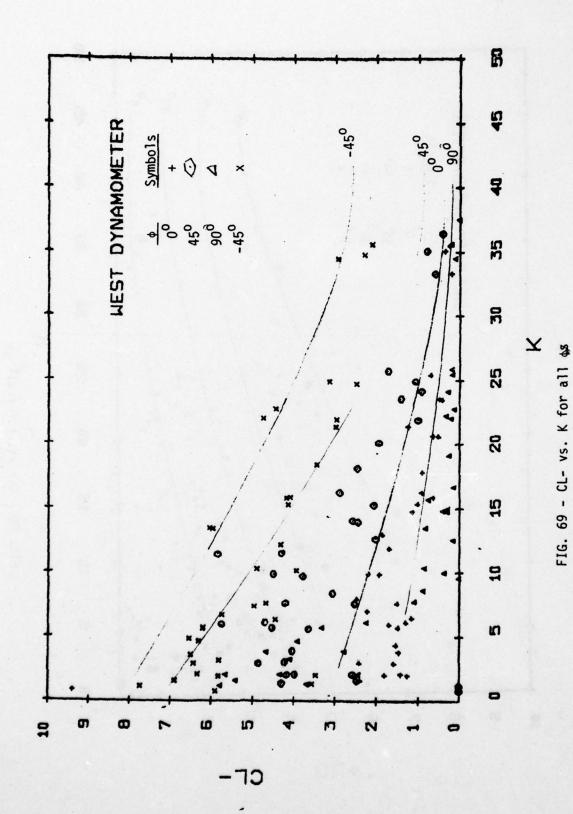




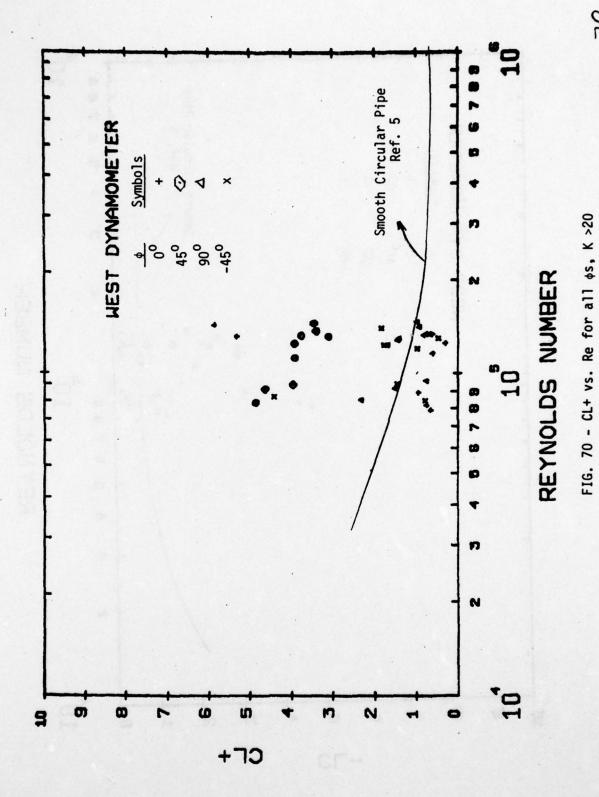


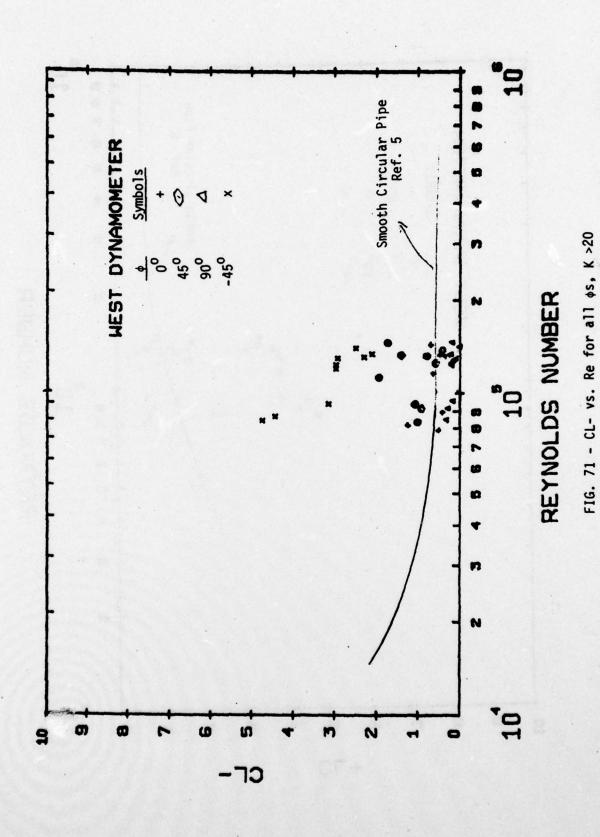












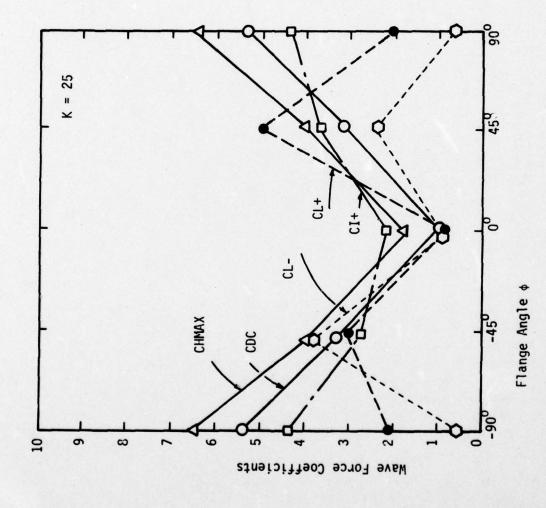


FIG. 72 - Force Coefficients vs.  $\phi$  for K = 25